INTEGRATION OF PRECISE POINT POSITIONING AND REDUCED INERTIAL SENSORS SYSTEM

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A dissertation

presented to Ryerson University

in partial fulfilment of

the requirements for the

degree of Doctor of Philosophy

in the Program of

Civil Engineering

Toronto, Ontario, Canada 2016

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ABSTRACT

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In Global Positioning System (GPS), Precise Point Positioning (PPP) achieves the highest accuracy in point positioning. It approaches centimetre-level accuracy in static mode and sub-decimetre accuracy in kinematic mode. PPP is an alternative approach to carrier-phase-based Differential GPS (DGPS) and offers advantages over DGPS. PPP uses GPS observations from a single receiver for position estimation, which is simpler than using more than one GPS receiver. However, PPP needs rigorous modelling for all errors and biases, which are otherwise cancelled out or mitigated when using DGPS. PPP's popularity is on the rise, as it is ideal for land-vehicle positioning and navigation. However, in challenging environments, PPP suffers from a signal loss that prevent continuous navigation or a reduction in the number of visible satellites that causes accuracy degradation.

This research integrates PPP with a Reduced Inertial Sensors System (RISS) — a low-cost system that uses data from reduced MEMS-based inertial sensors and vehicle odometry — to provide accurate and inexpensive land-vehicle navigation systems. The system is integrated in a tightly coupled mode through the use of an Extended Kalman Filter (EKF), which employs an improved error model for the RISS data. The system was tested using data from real driving routes with single-frequency code-based PPP/RISS (SF-code-PPP/RISS), dual-frequency code-based PPP (DF-code-PPP/RISS), smoothed dual-frequency code-based PPP (S-DF-code-PPP/RISS), and code- and carrier-phase-based PPP (code-carrier-PPP/RISS). The performance of the developed PPP/RISS was evaluated using position RMS and maximum errors during continuous GPS availability as well as during signal outages.

The developed integrated algorithms were assessed using three real road tests that capture different navigational conditions. The results show that when five or more satellites are available, code-carrier-PPP/RISS solution is superior to that of SF- and DF-code-PP/RISS. For latitude, code-carrier-PPP/RISS solution was 47% and 20% more precise than the SF- and DF-code-PP/RISS counterparts, respectively. For longitude, code-carrier-PPP/RISS solution was 65% and 31% more precise than the SF- and DF-Code-PP/RISS counterparts, respectively. Similarly, the altitude solution was improved by 46% and 25%, respectively. During GPS signal outages of 60 seconds, code-carrier-PPP/RISS's algorithms outperformed that of SF- and DF-code-PPP/RISS by about 35% when the satellite availability level was set to three satellites. For other satellite availability levels, the algorithms performed almost identically.

AKNOWLEDGEMENTS

I would like to acknowledge and thank the NSERC and OGS, for granting me very precious scholarships. Also my appreciations goes to: Geodetic Survey Canada of Natural Resources Canada (NRCan) for providing the source code of the traditional un-differenced GPSPace PPP software, and NovAtel Inc. for providing for some time the NovAtel IMU-CPT unit.

I would like to express my gratitude and appreciation to my long-term academic supervisor Dr. Ahmed El-Rabbany for his encouragement, guidance, and support throughout my undergraduate and graduate studies at Ryerson. The enthusiasm and knowledge conveyed in his teaching was one of the original reasons I choose Geomatics and pursued graduate studies. It was my great pleasure working under his supervision.

I would like to express my thanks and gratitude to my academic co-supervisor, Dr. Abdelmageed Noureldin, for his supervision, encouragement, valuable suggestions, and guidance throughout my PhD research. His abundant cooperation and understanding deserve unbounded appreciation.

I also would like to extend my thanks to Dr. Tashfeen Karamat, from Royal Military College for his kindness, support and guidance during data collection sessions. He was both available and extremely helpful answering questions I had regarding my research data.

In addition, I wish to extend my appreciation and thanks to my PhD defense committee members, Dr. Mostafa Hefnawi, Dr. Bassam Jubran, Dr. Songinan LI, Dr. Ahmed Abdelrahman, and Dr. Julia Spaniol for dedicating their valuable times to review my thesis.

Finally in alphabetic order, many thanks to: Dr. Akram Afifi, Ashraf Elshorbagy, Dr. Imad Khalafalla, Kamil Faisal, Salem Morsy .and Dr. Wai Yeung Yan, my colleagues in the Department of Civil Engineering, for the nice moments, friendship, and support I received throughout the entire course of my study.

DEDICATION

To my parents, Habiba, my wife, Daniyah, Elobeid, my sisters and brothers.

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ABBREVIATIONS

AFS Atomic Frequency Standards

AI Artificial Intelligence

ANFIS Adaptive Neural Fuzzy Inference Systems

ANN Artificial Neural Networks

C/A code Coarse/ Acquisition

Code-Carrier-PPP/RISS Code and Carrier PPP/RISS

CODE Centre for Orbit Determination in Europe

DD Double Difference

DF-Code-PPP/RISS Dual-Frequency Code PPP/RISS

DGPS Differential Global Positioning System

DoD Department of Defence

DOP Dilution of Precision

DR Dead-reckoning

ECEF Earth Centered Earth Fixed

ECI Earth-Centered Inertial System

EKF Extended Kalman Filter

ENU East North Up

FOG Fiber-optic Gyroscope

GDOP Geometric Dilution of Precision

GIM Global Ionosphere Maps

GLONASS Global Navigation Satellite System “GLObalnaya NAvigatsionnaya Sputnikovaya Sistema”

GNSS Global Navigation Satellite Systems

GPS Global Positioning System

IE Inertial Explorer

IF Ionospheric Free

IGS International GNSS Service

IMU Inertial Measurement Unit

INS Inertial Navigation System

IONEX IONosphere Exchange format

ISA Inertial Sensor Assembly

JPL Jet Propulsion Laboratory

KF Kalman Filter

LC Linear Combination

LKF Linearized Kalman Filter

LLF Local Level Frame

MCS Master Control Station

MEDLL Multipath Elimination Delay Lock loop

MEMS Micro-Electro-Mechanical Systems

MSE Mean Square Error

NAVSTAR NAVigational System using Timing and R

NMF Niell Mapping Function

NOAA National Oceanic and Atmospheric Admin

NOAATrop NOAA Tropospheric Model

NRCan Natural Resources Canada

NWP Numerical Weather Prediction

OBD On Board Diagnostics

P-code Precise

PDF Probability Density Function

PDOP Position Dilution of Precision

PF Particle Filter

PPP Precise Point Positioning

PRN Pseudo-random noise

RBA Resonating Beam Accelerometers

RINEX Receiver-Independent Exchange For

RISS Reduced Inertial Sensor System

RLG Ring Laser Gyroscopes

RMS Root Mean Square

RRSD Receiver-Receiver Single Difference

RTK Real Time Kinematic

SDF-Code-PPP/RISS Smoothed Dual-Frequency PPP/RISS

SD Single Difference

SF-Code-PPP/RISS Single-Frequency PPP/RISS

SNR Signal to Noise ratio

SPAN Synchronized Position Attitude Navigation

SSD Satellite-Satellites Single Difference

STEC Slant Total Electron Content

TC Tightly-Coupled

TEC Total Electron Content

UKF Unscented Kalman Filter

UNB University of New Brunswick

US-TEC United States Total Electron Content

UTC Ultra-Tightly Coupled

VMF1 Vienna Mapping Function 1

VTEC Vertical Total Electron Content

WL Wide-Lane

Chapter 1

1 Introduction

The integration of Global Positioning System (GPS) and Inertial Navigation System (INS) often refers to the data fusion of measurements obtained from a GPS receiver and inertial sensors (accelerometers and gyroscopes). GPS/INS integration is formulated as a state-space estimation problem where the Kalman Filter (KF), or a modified version of it, is applied to obtain suboptimal solutions. In many applications, GPS provides positioning information with consistent and acceptable accuracy when a GPS receiver gets signals from four or more GPS satellites (Grewal et al., 2007). However, GPS suffers from problems such as signal outage and multipath, which degrades its accuracy or even makes the system useless. In order to look for a continuous positioning solution, integration of GPS and INS is one of the available viable solutions. INS is an autonomous system that is immune to external interference; however, its accuracy decreases in the long term because of the sensor's bias error drift, scale factor instability, and misalignment (Gleason and Gebre-Egziabher, 2009).

Integrating GPS and INS benefits from each system's advantages such that the integration overcomes drawbacks of each individual system; the INS reaps the merits from the accuracy of GPS, while GPS benefits from the INS continuity. The most commonly used estimation technique for GPS/INS integration is KF. KF showed adequate performance in handling linear INS errors, especially when high-grade (IMU) is used.

The use of micro-electromechanical system (MEMS)-based IMU reduced the GPS/INS solution cost significantly. However, this solution's performance degrades very quickly in contrast to

higher grades of IMUs. This fact negatively impacts the performance of the MEMS-based GPS/INS navigation solution during GPS outages where severe error growth in position occurs. The commonly used Linearized KF (LKF) and Extended KF (EKF) adopt linearized error-state models. According to Noureldin et al. (2009), KF techniques suffer from divergence during outages. This divergence is mainly due to approximations during linearization process and system "mis-modelling," especially when using MEMS-based IMUs. To enhance the performance of MEMS-based GPS/INS integration, non-linear estimation techniques that avoid models' linearization should be used. In this research, different advanced non-linear estimation techniques will be explored.

In GPS/INS integration, Differential Global Positioning System (DGPS) is usually employed to obtain relatively high accuracy. However, DGPS is logistically complex and relatively expensive. An alternative to DGPS is precise point positioning (PPP). Besides proper modelling of the errors that cancel out when using DGPS, PPP requires precise satellite coordinates and clocks. PPP substitutes the burden of carrying out field procedures that are otherwise needed in DGPS to mitigate GPS errors (Héroux et al., 2004). The PPP technique can offer several significant advantages to applications, compared to DGPS. First, PPP requires only one GPS receiver and, therefore, it removes the need for GPS users to set local base stations. Thus, it overcomes the spatial operating range limit as well as the constraint of simultaneous observations on both rover and base receivers involved by DGPS. The major advantage of using PPP is that decimetre-level accuracy can be achieved without the use of a base station, while its major drawback is that its solution needs longer convergence time if compared to DGPS. Long convergence time is mainly caused by inadequate modelling of residual GPS errors and the

difficulty in fixing carrier-phase ambiguities to integer values. PPP has to account for all GPS errors. Some of these error's sources, such as code and phase biases, are not simple to handle and model, as they tend to merge with ambiguity parameters during the estimation process, which leads to unsuccessful ambiguity resolution.

The major objectives of this research are to enhance the performance of single- and dual-frequency kinematic PPP and to investigate its suitability for uninterrupted land-vehicle navigation. To achieve these objectives, this research includes development of a new algorithm to integrate PPP and reduced low-cost inertial navigation sensors.

1.1 Literature Review

1.2 Developments in PPP Research

Witchayangkoon (2000) investigated PPP using dual- and single-frequency pseudorange and carrier phase observations. Both static and kinematic modes were investigated. For observation with low multipath, results show that single-frequency ionosphere-free PPP solutions are equivalent to the dual frequency solutions. Witchayangkoon (2000) reported that using tropospheric information does not appear to improve dual-frequency PPP solutions as compared to a case in which a tropospheric parameter is estimated to be an unknown. However, a priori tropospheric information seems to benefit single-frequency kinematic PPP. In terms of solutions accuracy, Witchayangkoon (2000) reported 10 cm when static solutions compared with published coordinates and around one metre in kinematic mode. In other research, Gao and Chen (2004) presented sub-decimetre levels of accuracy after using the real-time PPP method with online satellite orbit and clock products.

In order to separate satellite and receiver biases, Leando and Santos (2006) introduced an approach called wide area–based PPP. The main idea of this approach is to determine satellite's fractional biases using a network of receivers. They applied the estimated biases to a single receiver within the same network's area. Also, in order to separate fractional biases from other parameters — such as ionospheric delays and ambiguities — a de-correlation filter was used. In experiments carried out involving nearby stations, including stations sharing the same GPS antenna, the de-correlation filter was used to determine ionospheric delays. These delays were found to agree very well with each other. Data from seven consecutive days of observations taken from an IGS station were processed to determine the differential receiver-satellite fractional biases. Ambiguities and fractional biases were determined for each passage of PRN20 on L1/frequency. The results showed that, although the ambiguity values are different for each passage, with differences of several meteres, the fractional biases seemed to be stable with a mean and standard deviation of about –0.6 cycle (3.9 cm) and 0.2 cycles, respectively.

Wang and Gao (2007) investigated the existence, property and determination of the GPS receiver initial phase bias. They examined different GPS satellites and receiver conditions including receiver restart, satellite rise/fall, and signal full loss of lock. They also investigated the use of initial phase bias calibration to support the ambiguity resolution in PPP. They proposed a method that includes two major steps. The first step employed two GPS receivers in a zero-baseline configuration mode to eliminate satellite-related errors, including the satellite initial phase bias, so that the differential receiver initial phase bias could be estimated. The second step estimated the integer ambiguities on both L1 and L2 frequencies using the Geometry-Free (GF) phase combination and the resolved wide-lane ambiguity. In assessing the reliability of the proposed

method, the resolved ambiguity was validated by comparing the estimated ambiguities with the double difference ambiguity obtained from the rounding of the zero-baseline. Then, after the removal of the resolved integer ambiguity component from the GF observation, the inter-frequency receiver initial phase bias was determined. To improve bias determination, the GPS observation data was de-noised using the Wavelet technique. Their results from using GPS receivers from two different manufacturers confirmed the existence of the initial phase bias and also identified its instability in response to receiver restart and full loss of lock. The satellite initial phase bias can be calibrated since it is stable for the long term.

Ge et al. (2008) showed that the Un-calibrated Phase Delays (UPDs) are fairly stable in time and position and can be estimated with high accuracy and reliability through a statistical analysis of the ambiguities estimated from reference network. They estimated the fractional parts of the Single-Difference (SD) UPDs between satellites in wide-lane and narrow-lane from a global reference network. By using the obtained SD UPDs as corrections, the SD ambiguities can be fixed to its integer values. With data from a dense network of 450 IGS stations, the efficiency of the ambiguity-fixing strategy was validated. They concluded that, on average, more that 80% of the independent ambiguities could be fixed reliably. The work done in Ge et al. (2008) is for static PPP; however, it can be applied in future kinematic PPP research.

Banville et al. (2008) investigated satellite and receiver phase biases calibration for undifferenced ambiguity resolution. In this research, a receiver phase-bias calibration technique using a GPS simulator was proposed. A simulator was used to generate errorless signals, which are ideal to isolate biases inherent in the receiver. Results showed that this calibration technique is complex. This complexity is mainly due to the correlation between the receiver clock and

ambiguity parameters. As such, eliminating receiver phase biases through between-satellite single differencing still appears to be ideal. In regard to satellite phase bias calibration, a wide-lane phase-bias calibration method was proposed. This proposed method was shown to be coherent with PPP's functional model.

Colombo et al. (2004) used recursive KF and smoother estimator to process static and kinematic dual frequency GPS observations to obtain static and kinematic PPP solutions. To eliminate receiver clock, single-difference between-satellite was carried out. Precise satellite orbits and clocks from the IGS were used, while estimating receiver's coordinates ionosphere-free carrier phase LC biases and tropospheric refraction model errors, phase wind-up, relativity, satellite and receiver antenna corrections for Earth tides, ocean loading, and pole tides. In their conclusion, they reported that the convergence time in kinematic mode was typically 30-40 min and the accuracy in the order was less than 10 cm.

In attempts to achieve undifferenced ambiguity resolution, Collins et al. (2010) developed a model known as the de-coupled clock model. This model allows for satellite hardware delays and satellite carrier-phase initial phase bias to be lumped to the GPS satellite clock corrections. In this work, only static PPP data was processed; however more work is required to investigate the impact of kinematic data.

In attempts to shorten the PPP convergence time, Elsobeiey and El-Rabbany (2010a) used global cluster stations to estimate second-order ionospheric delay. Findings from this research showed that accounting for the second-order ionospheric delay and applying the National Oceanic and Atmospheric Administration (NOAA) tropospheric corrections can improve the final PPP coordinate solution by about 3 mm and improve the convergence time of the estimated parameters

by about 15%. In Elsobeiey and El-Rabbany (2010b), results showed stochastic modelling of GPS observables for specific GPS receiver type. A 40% reduction in the PPP solution convergence time was shown to be possible, especially for the horizontal component, by proper modelling of the stochastic characteristics of the modernized signal. Also, Ibrahim and El-Rabbany (2009) implemented the NOAA tropospheric corrections along with the Vienna mapping function 1 (VMF1). Both are based on weather prediction models, into PPP solution, findings of this research showed positive results in terms of reducing the convergence time and improving the position solution.

1.3 Integration of autonomous GPS with INS

The integration of autonomous GPS with INS has been extensively investigated in the last two decades as it has several benefits, such as improved cycle slip detection (Bisnath and Gao, 2009).

At Applanix Corporation, Shin and Schenzinger (2009) investigated the effect of aiding PPP using high-grade INS on PPP convergence time during GNSS outages. They compared PPP/INS in both loosely and tightly coupled schemes. When the outage durations were within 30 seconds, the IAPPP in tightly coupled schemes was still able to meet decimeter-level accuracy. In the loosely coupled scheme, for outages longer than 10 seconds, the accuracy was at the meter level. In this research, data for both land vehicles and airborne vehicles were used.

In Zhang and Gao (2008), a tightly coupled PPP/INS model was presented. To test the model performance, airborne data was used. Results of this research showed that position and attitude accuracies are comparable to those of DGPS/INS current systems. However, these results are based on one data set, and do not show details about accuracy during GPS outages.

El-Diasty (2015) used a PPP-based GPS/INS integration system in hydrographic surveys performed by multi-beam echo-sounder. In this work, El-Diasty claimed that the PPP-based GPS/INS integration solution accuracy for horizontal position, roll, pitch, heading and heave can fulfills the International Hydrographic Organization (IHO) standards at 95% confidence level. El-Diasty (2015) claimed that PPP-based GPS/INS integrated system can be alternative inexpensive navigation solution to RTK-based GPS/INS integrated system.

Kjørsvik et al. (2010) used a tightly-coupled PPP/INS integrated system in car navigation. Results showed the difficulties of PPP in environments with frequent GNSS outages. GPS outages prevent proper convergence of the carrier phase bias estimates, and inaccuracies of code phase measurements significantly affects the INS navigation states. When a loosely coupled PPP/INS results shows no significant improvement over a pure PPP solution. Position accuracies for tightly-coupled is reported as a few decimeters with an improvement an improvement of 30–40%.

Gao et al. (2015) used ionosphere constrained (IC) PPP model instead of ionosphere-free PPP in a tightly coupled integration of GPS and INS. The tightly-coupled integrated IC-PPP is tested by using data from an airborne and a vehicle-borne experiment. Gao et al. (2015) claimed that the results confirm that IC-PPP/INS could improve the performance of the LC-PPP/INS and IC-PPP in position and velocity, for land vehicle when GPS is available, as 10 cm and a few mm/s of RMS error respectively. During GPS outage on average, IC-PPP/INS could improve the accuracy of IC-PPP by 57%, 78% and 67% and that of LC-PPP/INS by 21%, 11% and 23% in the north, east and vertical components, respectively. In other words, IC-PPP/INS can significantly improve the position accuracy during the re-initialization process and reduce the re-convergence

time. In this work the authors also claim that IC-PPP/INS also shows a better initialization performance than that of LC-PPP/INS.

Crespillo et al (2014) used a GNSS/IMU integrated system and a track map a precisions of less than a meter have been obtained and the track selection has been decided correctly all the time. Results also show that reliable position may still be reachable with only two or three visible GNSS satellites. When loosely-coupled and tightly-coupled integration are compared, result has shown that even in normal visibility scenarios, the tightly coupled integration is more robust to the existence of outlier satellite signals and to the non-modeled errors. In all the assessments conducted in this work, the satellite geometry was of high significance.

Karamat et al (2014) examined the feasibility of integrating DGPS code and carrier measurements integrated with a reduced set of vehicle's sensors through an extended Kalman filter (EKF) employing tightly coupled integration scheme. The performance of the integrated system was compared, using carrier-phase based reference, with two similar integration schemes employing undifferenced GPS measurements, where atmospheric effects are corrected using either Klobuchar model or ionosphere-free linear combination. The performance of the system in real road tests showed in 2-D positioning, the integrated system (DD EKF-based) performed 46% superior than single-frequency code EKF-based and 21% better than IF EKF-based. In altitude, EKF-DD showed 66% improvement over single-frequency code EKF-based and 14% over IF EKF-based. In GPS outages, the overall performance of EKF-DD system was 21% and 10% better than single-frequency code EKF-based and IF EKF-based, respectively.

1.4 PPP Limitations and Challenges

Limitations and challenges of PPP include:

- the convergence time after initialization or re-initialization right after a GPS signal outage.
- that the combined code and phase noise is amplified compared to the noise of isolated signals.
- integer characteristics of the phase ambiguities get lost and ambiguity fixing is prevented.

1.5 Current State of the Art in the Area of GPS/INS Integration Techniques

There are three categories of estimation techniques used to integrate GPS and INS: KF, Artificial Intelligence (AI), and sampling-based filters. The sampling-based category includes Unscented Kalman filters (UKF) and Particle Filters (PF).

KF is an optimal estimator that provides a sequential recursive algorithm suitable for linear systems. KF technique is still considered to be the benchmark for GPS/INS integration (Dissanyake and Sukkarieh, 2001; Oberdovic et al., 2007). However, it suffers divergence during GPS outages because of approximations, linearization and mis-modelling, especially when MEMS-based inertial sensors are used (Noureldin et al., 2009). KF is undermined because of the following inadequacies (Noureldin et al., 2009):

1) It requires stationary stochastic modelling of sensor errors, which may not be applicable to the environment of low-cost MEMS devices. In general, KF is ideal for estimating the error state of any linear system driven by white Gaussian noise (stationary random process with unit covariance). However, because of non-stationary random errors, it is usually difficult

to set a certain stochastic model for each inertial sensor that works efficiently and reflects the long-term behaviour of sensor errors in all environments (Brown and Hwang, 1992).

2) KF requires *a priori* information of the system and measurement covariance matrix for each new sensor. It takes a long design process to tune corresponding parameters. Furthermore, unstable error features of MEMS sensors make tuning parameters that best fit *a priori* information in one time may not keep up their performance at another time.
3) KF's accuracy depends on external aid. Therefore, it provides relatively poor performance during long GPS outages.
4) In KF, the weak observation of some error states may lead to unstable estimates. For instance, the weakly observable errors might be optimally estimated by KF in the long term only. This makes the convergence of their error covariance very slow, thus disturbing the stability of KF prediction of other correlated errors (Noureldin et al., 2002)
5) As a time domain recursive filter, KF highly depends on short-term statistical characteristics of data. A set of KF parameters that resulted in good estimation of error states for one data set may not result in good estimation of errors for another data set (Noureldin et al., 2007a). In other words, KF is so sensitive to local statistics that it may result in weak predictions if there is any short-term mis-modelling.

In order to overcome KF limitations, non-linear techniques were investigated in the last few years by many researchers. These non-linear techniques are based on: (1) AI; (2) UKF; and (3) PF.

For AI-based techniques, ANN has been widely used for GPS/INS integration. ANN is an interconnected assembly of simple processing elements, units, or nodes (known as neurons) whose

functionality is mimicking the human brain. The processing ability of the network is stored in the neuron connections weights, obtained by adapting to, or learning from, a set of training patterns (Gurney, 2003). ANN was developed and utilized in various schemes to totally replace KF and provide reliable navigation solutions for higher grade IMUs (Chiang and El-Sheimy, 2002; Shin, 2005; Chiang el al, 2006). There are different techniques for implementing ANN in GPS/INS.

The Multi-Layer Perceptron (MLP) neural networks have been suggested for and applied to different types and grades of INS (Noureldin et al., 2004). A Position and Velocity Update Architecture (PVUA) utilizing two MLP networks can process the INS azimuth and velocity to provide the position components along both east and north directions (Chiang et al., 2006). In this research the parameters of the MLP networks were adapted using GPS position and velocity updates. However, the neural network model of PVUA dealt with the INS position components instead of their errors. As a result, no information about the accuracy of this system could be decided during the navigation task. Moreover, besides the fact that no sensitivity analysis was provided for changing the internal structure of the MLP networks on the system performance (i.e., the number of hidden layers and number of neurons in each layer), the real-time implementation and the accuracy of the system during this mode of operations was not addressed.

Semenuik and Noureldin (2006) suggested an AI-based Segmented Forward Predictor (ASFP). ASFP was used to process segments of INS and GPS position and velocity data using Radial Basis Function Neural Network (RBFNN) to provide future prediction of INS errors. Although effective for both tactical and navigational grade INS, one limitation of the ASFP technique was the virtual extension of GPS outages due to the nature of GPS and INS data

segmentation. Moreover, the ASFP method exhibited inadequate performance when applied to MEMS-based INS/GPS.

Different ANN and neuro-fuzzy methods for GPS/INS integration were introduced. However, these methods provided relatively poor positioning accuracy during long GPS outages. Moreover, the internal system parameters had to be tuned during the navigation mission to reach the desired positioning accuracy (Noureldin et al., 2007b). To overcome these limitations, Noureldin et al. (2007b) implemented the adaptive neuro-fuzzy inference system (ANFIS) with DGPS data for land vehicle applications. Also, real-time implementation issues of the ANN-based GPS/INS integration were addressed through, firstly, the AI-based segmented forward predictor (ASFP) (Semenuik and Noureldin, 2006), and secondly, the special weight update method (Chiang et al., 2004). However, despite these efforts, AI techniques still have limited capabilities when applied to low-cost navigation systems involving MEMS inertial sensors.

In an attempt to augment KF with ANN, Perreault et al. (2008) proposed an augmented KF/ANN system for the integration of MEMS-based inertial sensors with GPS. In this system two KFs are used along with a RBFNN. During GPS availability, the first KF, the main filter, is used to correct the mechanization output to obtain the estimated position, velocity, and heading. The second KF is used in prediction mode and its output as well as the mechanization and GPS output are passed to train the RBFNN. The purpose of the RBFNN is to correct for the non-linear parts of residual inertial errors that KF cannot correct. During GPS outages, KF works in prediction mode and its output is passed to the RBFNN to further correct for non-linear parts of the residue. As Perreault et al. (2008) claim, the final estimated navigation states after these corrections are

better than those corrected by KF prediction alone. However, issues like the time needed for the training routine for the RBFNN and its application in real time were not discussed.

As described in Sharaf and Noureldin (2007), the limitations of AI-based INS/GPS integration techniques are that:

1) Their input does not include any statistical information, nor do their output solutions include associated statistics.
2) They do not use well-developed mathematical models for vehicle motion and measurements.
3) Their performance is unacceptable if a vehicle experiences a dynamics range not included in the training set.

Generally, AI-based GPS/INS integration techniques provide inadequate performance when low-cost MEMS-based inertial sensors are used. The reason for this is the existence of significant bias errors and scale factor instabilities in the sensor's raw data. Another limitation, because AI-based GPS/INS integration techniques lack the use of models for INS sensors' errors, is the INS-significant non-linear and non-stationary types of errors. So, new advanced techniques are still need to improve the performance of error mitigation.

In attempts to provide robust Positioning/Navigation (POS/NAV) solutions using low-cost MEMS-based inertial sensors, more advanced estimation techniques have been explored. Research has been conducted to overcome limitations — i.e., the linearization of motion and linear stochastic models' equations that accompany the use of KF when it used to estimate MEMS-based sensors' errors. Techniques that avoid linearization have been investigated, which lead to the use of

sampling-based techniques such as UKF and PF. These techniques are different from KF, as they tend to provide a numerical approximate solution to an exact non-linear problem rather than providing the optimal solution for an approximate problem. These techniques are based on probable solutions that estimate the Probability Density Function (PDF) of the state conditioned on the set of measurements instead of providing a single best estimate. In KF, a Gaussian PDF is assumed and propagated in a parametric form using the mean and covariance, while in UKF and PF this PDF is approximated using a set of samples.

UKF uses a fixed number of deterministically selected sample points called the sigma points (SP) to approximate the PDF (Shin, 2005). These SP capture the true mean and covariance of the PDF. When these SPT are propagated through the non-linear system, they capture the transformed mean and covariance accurately to the second order of any non-linearity, where, in the case of LKF and EKF, the mean and covariance are calculated accurately to the first order with all higher-order terms truncated (Shin, 2006).

Unlike UKF, PF uses a set of random samples or particles. It is a universal density approximator. PF can be defined as a hypothesis tracker that approximates the filtered posterior distribution by a set of weighted particles. It weighs particles based on a likelihood score and then propagates these particles according to a motion model. PFs carry out sequential Monte Carlo (SMC) estimation based on particle representation of probability density (Yi and Grejner-Brzezinska, 2006a). There are two steps in PF design: (1) sequential importance sampling and (2) resampling. The major drawback of PF is that it is computationally expensive. With improvements in computer capabilities, PF gained popularity among researchers dealing with non-linear modelling and estimation.

In the area of GPS/INS integration, PF was used to include elements of non-linearity and non Gaussianality in motion dynamic and noise distributions. Carvalho et al. (1997), for a tightly coupled integration of simulated data, used non-linear modelling of GPS observations, while using a linearized INS error state and random walk for inertial sensor stochastic errors. In Garvalho et al. (1997), Yi (2007), and Aggarwal et al. (2007), the importance of density for PF was obtained using extended KF (EKF). The results for the combination of EKF and PF (EPH) did not show significant improvements over the EKF results. Furthermore, the results presented by Yi and Grejner-Brzezinska (2006a) were for high-grade IMU, not for MEMS-based IMU. In Yi and Grejner-Brzezinska (2006b), the MEMS-based IMU results for EPH were sometimes worth more than that for EKF alone. The results of the work presented by Aggarwal et al. (2007) using EKF and PF showed no considerable enhancement over results obtained by EKF. An adaptive scheme of EPH was developed by Aggarwal et al. (2008) and Aggarwal et al. (2009). It switches between EPF and EKF. It uses the EKF during stops and the EPF during relatively higher-dynamic environments. Also, results obtained for using this adaptive scheme showed small improvement over using EKF.

In Georgy et al. (2009) and Georgy et al. (2010), mixture PF, as an enhanced version of PF, was used with INS measurements coming from reduced inertial sensor system (RISS) instead of full IMU. While the sampling/importance resampling (SIR) PF samples from the prior importance density and the likelihood PF samples from the observation likelihood, the mixture PF samples from both densities, then appropriate weighting is achieved followed by resampling. This mixture of importance densities leads to a better performance. The performance of this method is examined by road test trajectories in a land vehicle and compared with KF.

1.6 Motivation

Development in electronics makes it easy for consumers to have low-cost, powerful navigation equipment in a convenient weight and size. This type of navigation equipment, like GPS receivers and MEMS inertial sensors, is useful. In reducing the cost, precise positioning PPP is used instead of DGPS. Therefore, integrating autonomous GPS and INS can benefit many applications, such as:

1) Land vehicle navigation: Currently car navigation relies mainly on GPS; however, GPS suffers from signal blockage in downtown cores, which can make the service not continuous and not reliable. The alternative is to integrate GPS with INS. The high cost of navigation and tactical grade INS prevent their utilization in low-cost navigation applications. Nowadays, auto industry competition is pushing automobile manufacturers to implement reliable low-cost GPS/INS systems in land vehicles.

2) Machine control: To locate, control, and automate machines and equipment in a construction or mining site is needed as a positioning aid. Nowadays, GPS systems are integrated into the design of bulldozers, excavators, skiffs, pavers, and motor graders. The need for machine control in various applications that require more accuracy and efficiency makes machinery manufacturers search for low-cost and accurate positioning systems.

3) Precision farming: The use of GPS in farming enables good management of resources and time. GPS has enabled farmers to accurately direct their equipment's movement, providing precise positioning for all equipment actions and chemical applications. Therefore,

implementation of continuous, low-cost, accurate positioning systems will continue to enhance agricultural processes.

4) Cooperative Vehicle Safety System (CVSS): This is a system for locating vehicles and enabling them to exchange their position with surrounding vehicles. CVSS consists of three components, namely: positioning systems, communications protocols and embedded maps. The purpose of CVSS is to alarm drivers when vehicles are too close to them and also for those they cannot see in their blind spots, in order to reduce chances of car collisions, especially while changing lanes. Nowadays, automobile manufacturers are targeting implementation of CVSS with restricted specifications in terms of high accuracy and low cost.

1.7 Objectives

The major objectives of this research are, firstly, to develop methods and algorithms for reliable MEMS-based autonomous GPS-aided INS positioning modules that enhance overall system accuracy and enable robust and accurate positioning information during GPS signal outages; and, secondly, to implement techniques for PPP error mitigation in order to enhance the accuracy of PPP and reduce convergence time. The targeted applications of this research are low-cost navigation solutions appropriate to commercial land vehicles, unmanned autonomous vehicles, machine control, and precision farming. The research focuses on three issues, namely: (1) the use of EKF to account for system non-linearity, instead of using KF, (2) the use of single-frequency PPP, and (3) the use of dual-frequency PPP instead of DGPS in applications that require high-accuracy solutions.

To achieve the major objectives of this proposal, the following tasks are addressed:

- investigating and implementing methods for reducing PPP convergence time, which includes proper modelling of hardware delay and initial phase bias
- investigating and developing methods that benefit from the INS aid to maintain good accuracy during GPS signal blockage
- investigating different non-linear estimation techniques to improve GPS/INS's overall accuracy
- investigating how to exploit combinations of accelerometers, gyroscopes and speed odometers.

1.8 Thesis Outline

This thesis is organized as follows:

Chapter 1 provides an introduction and discusses previous studies and limitations, the motivation for the thesis, its objectives and outline, and its research contributions.

Chapter 2 reviews the GPS background necessary to support this research. It addresses GPS signal structure, observables, and positioning techniques with emphasis on PPP. In this chapter various GPS measurement errors and the ways to account for them are also discussed.

Chapter 3 is devoted to an INS overview, including INS errors and ways to account for them. In this chapter emphasis is given to RISS.

Chapter 4 deals with the developed PPP/RISS models. It presents single- and dual-frequency PPP/RISS. This chapter also covers the EKF technique for solving different PPP/RISS models.

Chapter 5 presents different results obtained through varying navigation conditions. Position results, from the developed PPP/RISS integrated system, are extensively studied in all cases, particularly when there is GPS outage.

Chapter 6 presents the summary and conclusions of this research, as well as suggesting recommendations for future Chapter 7 presents the summary and conclusions of this research, as well as suggesting recommendations for future research.

Appendix A devoted for review of different coordinate systems that are utilized in this thesis. It also discusses transformation methods between these coordinate systems.

Appendix B gives a tabulated list of speculations of the major equipment, mainly IMUs and GPS devices, used for data acquisition for this research.

Appendix C devoted to the presentation of quality check reports generated by TEQC software to examine the GPS data before integrating it with the INS data.

1.9 Thesis Contribution

Contributions from this thesis can be listed as:

- Integrating GPS precise point positing and reduced inertial sensors system.
- Developing algorithms to integrate single-frequency code, dual-frequency code, smoothed dual-frequency code, code and carrier precise point positing and reduced inertial sensors.
- Using extended Kalman filter in integrating PPP and RISS.
- Employing PPP/RISS integrated system into land-vehicle navigation

- Testing the developed algorithms in urban and suburban navigation environments.

Chapter 2

2 Global Positioning System

This chapter presents an overview of the Global Positioning System (GPS) and its signal structure and positioning modes. The chapter focuses on Precise Point Positioning (PPP), with emphasis on its errors and how to mitigate them. PPP mathematical models are also presented.

2.1 GPS Overview

GPS is a satellite-based navigation system that was originally developed by the United States Department of Defense (DoD) in the early 1970s. GPS was initially developed as a military system to fulfil military requirements, but was later made available for civilian use and has since become a dual-use system that is accessible by both military and civilian users (El-Rabbany, 2006). GPS is capable of providing position, velocity, and time information for any user on or near the surface of the Earth, under all weather conditions where there is a direct line of sight between the user's receiver and four satellites or more. The minimal constellation of the system is 24 satellites distributed evenly on six orbits, each with an inclination of 55° on the equatorial plane, and an altitude of about 20,200 km above the surface of the Earth. GPS consists of three segments: the space segment, the control segment, and the user segment (Hofmann et al., 2001).

2.2 GPS Signal Structure

GPS satellites transmit a microwave radio signal centred on the L-band carrier frequency of the electromagnetic spectrum. The carrier frequencies are known as the L1 signal with a frequency of 1575.42 MHz and the L2 signal at a frequency of 1227.60 MHz, as well as the new L5 signal with a frequency of 1176.45 MHz. The satellite signal consists of the three L-band carrier frequencies, the ranging codes modulated on these carrier waves, and the navigation message. GPS signal is derived from the fundamental frequency f0=10.23 MHz, generated by the satellite atomic clock. Atomic clocks are the key to the GPS accuracy; they are based on atomic frequency standards (AFS) that produce reference frequency by stimulated radiation (Hofmann-Wellenhof et al., 2008). The L1 and L2 carrier frequencies are generated by multiplying the fundamental frequency by 154 and 120, respectively. Their corresponding wavelengths are approximately 19 cm and 24 cm, respectively. The signals contain codes that identify each satellite, time of the emitted signal, and satellite PRN number.

2.3 Positioning Modes

There are two general GPS modes to obtain position: relative or differential positioning and point positioning.

2.4 Relative positioning

Relative positioning is a more accurate positioning mode compared to point positioning. In Differential Positioning GPS (DGPS), two GPS receivers are employed to track the same satellites simultaneously. One of the two receivers, known as the base, is set at a reference point with

precisely known coordinates. The other receiver, known as the rover, is set at a point with unknown coordinates (El-Rabbany, 2006). In DGPS, mitigation of errors and biases is done through differencing measurements of the two receivers. The shorter the range between receivers is, the more similar the errors and biases. As such, for those receivers, a significant portion of the GPS error budget can simply be removed by combining their GPS observables. DGPS have been applied for precise positioning since they cancel out receiver and satellite clock errors and significantly reduce atmospheric delay (Héroux et al, 2004). Unfortunately, as the range between receivers increases, the errors and biases at the base and the rover receivers become less correlated; i.e., they do not cancel out sufficiently through differencing. This results in unsuccessful fixing of ambiguity parameters, which in turn degrades DGPS accuracy. Another major drawback of DGPS is its dependency on the measurements or corrections from a reference receiver or network, so errors in reference station coordinates can be propagated to newly established stations. In addition, DGPS has disadvantages such as limited applicability area and an increase in logistical complications.

2.5 **Point positioning**

Point or autonomous positioning is a technique that involves using a single GPS receiver to capture measurements from four or more satellites with which to compute the position of a point on or above the Earth's surface. In point positioning, error and bias mitigation methods are modelling and estimation.

Point positioning can be classified into classical and precise point positioning. In classical point positioning, pseudo-ranges from four or more satellites are processed to compute the

receiver's antenna position. The broadcast ephemeris is used to compute the corresponding satellite coordinates, satellite clock corrections, and ionospheric delay. The expected accuracy of the classical point positioning is about 13 m at the 95% probability level (Hofmann-Wellenhof et al., 2008).

Precise point positioning (PPP) is a stand-alone positioning approach that utilizes dual frequency pseudo-range and carrier phase observations along with precise satellite orbit and clock products. PPP is used to achieve the highest possible accuracy of point positioning; it refers to centimetre positioning accuracy in the static mode and sub-decimetre accuracy in kinematic mode (Leick, 2004). PPP is an alternative approach to DGPS. Using the PPP technique with undifferenced GPS observations, positions can be estimated logistically more simply than using the DGPS technique (Banville et al, 2008; Bisnath and Gao, 2009). PPP requires proper modelling of the errors that cancel out in the case of DGPS. The PPP technique can offer several significant advantages to applications compared to DGPS. First, PPP requires only one GPS receiver and, therefore, removes the need for GPS users to establish local base stations. As a result, it eliminates the spatial operating range limit as well as the constraint of simultaneous observations at both rover and base receivers involved in DGPS. The major drawback of PPP is that its solution needs a longer convergence time compared to DGPS.

2.6 PPP Mathematical Models

Depending on the GPS observables used, a number of models are derived for PPP from the main GPS observation equations. These models include code-based, code, and carrier-phase.

2.7 GPS Observables

There are three main GPS observables: pseudorange, carrier-phase, and Doppler measurements. Code and carrier-phase observations can be modelled as follows (Leick, 2004; Teunessen, 1998 and Elsobeiey, 2012):

$$C_1 = \rho + c(dT - dt) + T + I_1 + c(d_{rC1} - d_{sC1}) + dm_{C1} + e_{C1} \quad (2.1)$$

$$C_2 = \rho + c(dT - dt) + T + I_2 + c(d_{rC2} - d_{sC2}) + dm_{C2} + e_{C2} \quad (2.2)$$

$$P_1 = \rho + c(dT - dt) + T + I_1 + c(d_{rP1} - d_{sP1}) + dm_{P1} + e_{P1} \quad (2.3)$$

$$P_2 = \rho + c(dT - dt) + T + I_2 + c(d_{rP2} - d_{sP2}) + dm_{P2} + e_{P2} \quad (2.4)$$

$$\Phi_1 = \rho + c(dT - dt) + T - I_1 + c(\delta_{r1} - \delta_{s1}) + \lambda_1[N_1 + \phi_{r1}(t_0) - \phi_{s1}(t_0)] + \delta m_1 + \varepsilon_1 \quad (2.5)$$

$$\Phi_2 = \rho + c(dT - dt) + T - I_2 + c(\delta_{r2} - \delta_{s2}) + \lambda_2[N_2 + \phi_{r2}(t_0) - \phi_{s2}(t_0)] + \delta m_2 + \varepsilon_2 \quad (2.6)$$

where P_1, C_1 are the pseudorange measurements on L1; P_2 and C_2 are the pseudorange measurements on L2; Φ_1 and Φ_2 are the carrier-phase measurements on L1 and L2, respectively; ρ is the geometric distance between the satellite antenna at the transmission time and the receiver antenna at the reception time; dT and dt are the receiver and satellite clock errors, respectively; dr and ds are frequency-dependent code delays for receivers and satellites, respectively; δr and δs are

frequency-dependent carrier phase delays for receiver and satellite, respectively; I_1 and I_2 are the ionospheric delays on *L1* and *L2* respectively; dm_1 and dm_2 are code multipath delays on *L1* and *L2,* respectively; δm_1 and δm_2 are carrier-phase multipath delays on L1 and L2, respectively; λ_1 and λ_2 are wavelengths on *L1* and *L2,* respectively; N_1 and N_2 are integer ambiguity for *L1* and *L2*, respectively; c is the speed of light in vacuum; $\phi r(t_0)$ and $\phi s(t_0)$ are frequency-dependent initial phases fractions in the receiver and satellite, respectively; eC_1, eC_2, eP_1, eP_2, ε_1 and ε_2 are relevant noise and unmodelled errors.

2.8 **Code-based PPP Model**

By elimination of the first-order ionospheric delay error through the ionospheric-free linear combination, code-based PPP models can be formed as:

$$P_{IF} = \frac{f_1^2 P_1 - f_2^2 P_2}{f_1^2 - f_2^2} = 2.546 P_1 - 1.546 P_2 \tag{2.7}$$

$$P_{IF} = \rho + c(dT - dt) + T + \frac{f_1^2}{f_1^2 - f_2^2} cd_{rP1} - \frac{f_2^2}{f_1^2 - f_2^2} cd_{rP2} + \varepsilon_{IF} \tag{2.8}$$

Here P_{IF} is the pseudorange ionosphere-free linear combination; ε_{IF} is the ionosphere-free linear combination noise, multipath, and other unmodelled errors like ocean loading, phase wind-up, and the Sagnac effect. Tropospheric delay is accounted for by using a suitable empirical model. f_1 and f_2 are the frequency of L_1 and L_2, respectively.

2.9 **Code- and Carrier-based PPP Models**

PPP with undifferenced dual frequency code and carrier-phase observations was proposed by Zumberge et al. (1997). Kouba and Héroux (2000) introduced the PPP technique using the traditional observation model, as follows:

$$P_{IF} = \rho + c(dt - dT) + T + \varepsilon_P \tag{2.9}$$

$$\Phi_{IF} = \rho + c(dt - dT) + T + N\lambda + \varepsilon_P \tag{2.10}$$

Here P_{IF} is the ionosphere-free combination of P_1 and P_2 pseudorange $(2.54P_1 - 1.546P_2)$; Φ_{IF} is the ionosphere-free combination of Φ_1 and Φ_2 pseudorange $(2.54\lambda\Phi_1 - 1.546\lambda\Phi_2)$; λ is the combination wavelength (= 10.7 cm); $\varepsilon_P, \varepsilon_\Phi$ are the relevant measurement noise components, including multipath.

Another model first introduced by Teunissen and Kleusberg (1998) uses an averaged code and carrier phase observations. It is also used at the University of Calgary by Gao and Shen (2001) and is known as the "U of C" model. The U of C model allows for the estimation of two float ambiguities for each satellite in addition to station coordinates, a receiver clock, and atmosphere parameters. The model relies on ionosphere-free linear combination, and is written as:

$$P_{IF/L_1} = 0.5(P_1 + \Phi_1) \tag{2.11}$$

$$P_{IF/L_2} = 0.5(P_2 + \Phi_2) \tag{2.12}$$

$$\Phi_{IF} = \frac{(f_1^2\Phi_1 - f_2^2\Phi_2)}{f_1^2 - f_2^2} \tag{2.13}$$

2.10 Doppler Measurements

The GPS signal transmitted from a satellite to a GPS receiver is not exactly L1 or L2 frequency; however, the frequency of the signal shifts when received at the receiver (Hofmann-Wellenhof et al., 2007). This is called the Doppler shift and it is exists because of the relative motion between the satellite and the receiver. The Doppler observation can be expressed by differentiating the GPS pseudorange equation. In Parkinson and Spilker (1996), the Doppler shift of the m^{th} satellite is defined as the projection of relative velocities of satellite and receiver onto the line of sight vector, multiplied by the transmitted frequency and divided by the speed of light — as follows:

$$D^m = \frac{[(V^m - V)]f_i}{c} \tag{2.14}$$

$$V^m = \begin{bmatrix} V_x^m \\ V_y^m \\ V_z^m \end{bmatrix} \tag{2.15}$$

$$V = \begin{bmatrix} V_x \\ V_y \\ V_z \end{bmatrix} \tag{2.16}$$

$$l^m = \frac{[(x - x^m), (y - y^m), (z - z^m)]^T}{\sqrt{(x - x^m)^2 + (y - y^m)^2 + (z - z^m)^2}} \quad (2.17)$$

where V^m is the m^{th} satellite velocity in ECEF frame; V is the receiver velocity also in ECEF frame; f_i is the satellite transmitted frequency; 1^m is the line of sight vector from the m^{th} satellite to the receiver; and c is the speed of light.

As mentioned above, Doppler observation can be expressed by differentiating the GPS pseudorange equation. So, when the measured Doppler shift D^m is given, the pseudorange rate $\dot{\rho}^m$ is calculated as follows:

$$\dot{\rho}^m = -\frac{D^m c}{L_1} \quad (2.18)$$

2.11 GPS Errors and Biases

The way to achieve high precision and accuracy positioning using GPS is with the resolution of the carrier-phase ambiguity to its integer values. Integer ambiguity resolution requires removing the correlation between the ambiguity parameters and all the other estimated parameters. To achieve high accuracy and short convergence time PPP, all GPS errors, GPS error residue, and biases — including hardware delay, ionosphere, and phase bias — must adequately modelled.

According to El-Rabbany (2006), there are several types of random and systematic errors that affect the accuracy of GPS observations. These errors can be classified as errors related to the GPS satellite, errors related to the GPS receiver, and errors related to the atmosphere. The satellite-

related errors include satellite clock errors, ephemeris errors, and satellite hardware delays. Receiver-related errors include receiver clock errors, receiver noise, receiver hardware delays, and antenna phase centre variation and multipath. Atmospheric errors include the effect of the ionosphere and the troposphere on the GPS signal.

2.11.1 Satellite orbital errors

Satellite orbital errors or satellite ephemeris are errors in the satellite's coordinates. Satellite orbital errors exist as a result of imperfect modelling of the forces acting onto the satellite (El-Rabbany, 2006). There are three sources of data that can be used to determine position and velocity vectors of a satellite in terrestrial reference frame at any time: almanac data, broadcast ephemerides, and precise ephemeris (Hoffmann, 2008). The data availability and accuracy are different for each of the above mentioned data sources (see Table 2.1) (Hoffmann, 2008).

Almanac data is aimed to provide the GPS user with adequate data to facilitate receiver satellite acquisition and for GPS surveying planning (Hoffmann, 2008). The broadcast ephemerides for a satellite are the predictions of the current satellite position and velocity as determined by the master control station (MCS). The broadcast orbital information is delivered in the form of Keplerian parameters along with their temporal variation (Hoffmann, 2008). These are uploaded to the GPS satellites, and transmitted to the user receiver in the navigation message. The precise ephemeris is the most accurate satellite orbit product. Precise ephemeris is comprised of post-processed values derived by, for example, the International GNSS Services (IGS), and is available to post-mission users via the Internet.

Ephemeris errors are a result of slight deviations in the actual orbital paths of the satellites from their predicted path. Broadcast errors are typically at the "few-metre" level while the precise ephemeris errors are at the decimetre-level. Ephemeris are largely mitigated by differential correction (in DGPS positioning) or in double differenced observables (formed from carrier phase measurements) when the surveyed baselines are not up to a few tens of kilometers. In very high precision applications and/or where the baseline lengths are hundreds or thousands of kilometres, residual ephemeris errors may limit the accuracy of the baseline solution.

Table 2.1 Uncertainties of Satellite Ephemeris (Adapted from Hoffmann [2008])

Ephemerides	Uncertainty	Remark
Almanac	Some kilometers	Depending on the age of data
Broadcast ephemerides	~ 1.0 m	Or even better
Precise ephemerides	0.05 – 0.20 m	Depending on the latency

In PPP, broadcast ephemeris must be excluded and precise ephemeris product must be used in the data processing instead. Precise ephemeris is produced by organizations such as the IGS, Jet Propulsion Laboratory (JPL), and Natural Resources Canada (NRCan). At present, precise ephemeris is available online with varied accuracy and latency. It is available as "rapid," "ultra-rapid" and "find." Table 2.2 summarizes the present (May 13, 2016) accuracy of the broadcast ephemeris and the availability and accuracy of the IGS precise satellite orbit and clock corrections (IGS, 2016).

Table 3.2 Accuracy and Latency of GPS Precise Orbit/Clock Products

(Adapted from IGS, 2016)

GPS orbit/Clock Product	Orbit accuracy	Clock accuracy	Latency	Sample interval
Broadcast	~ 100 cm	~ 5 ns (RMS) ~ 2.5 ns (STD)	Real-time	daily
Ultra-rapid (Predicted)	~ 5cm	~ 3 ns (RMS) ~ 1.5 ns (STD)	Real-time	15 min
Ultra-rapid (Observed)	~ 3 cm	~ 150 ps (RMS) ~ 50 ps (STD)	3 – 9 hours	15 min
Rapid	~ 2.5 cm	~ 75 ps (RMS) ~ 25 ps (STD)	17 – 41 hours	15 min 5 min
Final	~ 2.5 cm	~ 75 ps (RMS) ~ 20 ps (STD)	12 – 18 days	15 min 30 s

2.11.2 Satellite clock errors

GPS signals are generated onboard a satellite and are controlled by atomic clocks (cesium and /or Rubidium clocks). GPS atomic clocks are carefully monitored by the GPS Master Control Station (MCS) — however, these clocks are not perfect. The stability of these satellite atomic clocks, per day, is in the range of one to two parts in 1,013. According to the range of stability, satellite clock errors are in the range of 8.64 to 17.28 ns per day (El-Rabbany, 2006). Satellite clock errors can affect the range, between the satellite and the receiver, by 2.59 m to 5.18 m. In addition to clock errors, satellite clocks are also drifting over time. Satellite clock drifts are observed by ground stations and matched with master control systems that are combinations of more than 10 very

accurate atomic clocks. Satellite clock errors and drifts are modelled and transmitted as a part of the navigation message in the form of three coefficients of a second-order polynomial. The three coefficients are: satellite clock bias (a_0), satellite clock drift (a_1), and drift rate (a_2). The equation for the satellite clock error is:

$$dt^s = a_0 + a_1(t - t_c) + a_2(t - t_c)^2 \tag{2.19}$$

Here t is the observation epoch time and t_c is the satellite reference epoch time.

In computing the distance to satellites, GPS receivers subtract the satellite clock errors from the reported transmit time to come up with the signal travel time. For precise positioning such as PPP, the above equation is not sufficient to count for satellite clock error. The IGS precise clock product is used.

2.11.3 Ionospheric delay

The ionosphere is part of the Earth atmosphere that extends between an altitude of about 50 km and 1,000 km or more above the Earth (Hofmann-Wellenhof et al., 2008). The ionosphere contains ionized particles that exist as a result the Sun's ultraviolet radiation activities. The density of the ionized particles is not constant; it varies with altitude and accordingly can be arranged in four layers: D, E, F1, and F2 (El-Rabbany, 2006). The largest electron density appears in layer F2. The ionosphere creates delays in the GPS signal that are proportional to the total electron content (TEC) along the signal path. The ionosphere delays the code while it speeds up the carrier phase beyond the speed of light. In general, the ionosphere can cause a range in error of the order between 5 m to 15 m, and up to 150 m under extreme solar activities (El-Rabbany, 2006).

2.11.4 Ionosphere mitigation

Ionospheric delay is a major GPS error that must be accounted for to achieve accurate GPS positioning. Ionospheric delay is the largest possible error following using precise ephemeris and clock errors. Techniques for ionosphere mitigation depend on the GPS mode. In DGPS, ionospheric delay cancels out if the baselines are relatively short (less than 10 km) because the ionospheric error is highly correlated at both ends. In PPP, users of dual frequency data dual can cancel out the first-order ionospheric delay using the first-order ionosphere-free linear combination. Single frequency users, however, have to account for ionospheric error using: (1) specific empirical models such as Klobuchar model, (2) regional ionospheric models (e.g., United States Total Electron Content [US-TEC] maps), or global ionospheric models (e.g., IGS ionospheric maps [IONEX]), and (3) real time corrections from regional network (El-Rabbany, 2006). The US-TEC grids include both Slant TEC (STEC) and Vertical TEC (VTEC) for different locations and directions. US-TEC represents an accurate source of STEC because its accuracy is one to three TEC units at a spatial resolution of 1°×1° and a temporal resolution of 15 min.

2.11.5 Tropospheric delay

According to El-Rabbany (2006) the troposphere is lower layer of the atmosphere that extends to altitude of about 50 km from the surface of the Earth. Unlike the ionosphere, the troposphere is electrically neutral (non-ionized) and is a non-dispersive medium for radio frequencies below 15 GHz (Hay and Wong, 2000). The effect of the tropospheric delay is frequency independent (Hoffman el. Al, 2008). As a result, it creates similar delays on GPS carriers and codes. Tropospheric delay may be divided into two portions: dry and wet. The dry portion represents about 90% of the delay and can be predicted to a high degree of accuracy using mathematical

models and metrological measurements (temperature, pressure, and humidity) (Leick, 2004). The wet portion of the tropospheric delay is highly correlated with the water vapour along the GPS signal path and weakly correlated with the surface metrological measurements. Unlike the dry portion, the wet portion is not easy to predict. Several models are available to compute dry and wet zenith tropospheric delays. These include the: Saastamoinen model, Hopfield model, Modefied Hopfield model, Davis et al. model, Baby et al. model, NOAA Tropospheric model (NOAATrop), and University of New Brunswick (UNB) model. To obtain the slant tropospheric delay, a mapping function is needed. A number of mapping function models are available, which include the Niell mapping function (NMF), Davis mapping function, Chao mapping function, and Vienna mapping function (VMF1).

2.11.6 Receiver clock error

A GPS signal travels the distance between the transmitter (satellite) and receiver in a tiny fraction of a second. So, the ideal for measuring the signal travel time is to use very accurate atomic clocks at both the satellite and the receiver. Atomic clocks are very expensive to use at the receiver. Alternatively, GPS receivers are normally equipped with cheap and inaccurate clocks (crystal clocks) compared to satellite clocks. Crystal clocks induce much larger errors compared to satellite clocks. There are two ways of mitigating the receiver's clock error. One is applying the technique of differencing between satellites. Another is considering the receiver's clock error as one of the estimation process unknowns (El-Rabbany, 2006). The latter is the way followed in dealing with receiver's clock error in the PPP technique.

2.11.7 Multipath

Multipath is one of the major sources of error for GPS positioning (Leick, 2004). Multipath can happen at the receiver and/or at the satellite. At the receiver, multipath happens when the signal from the satellite reaches the receiver antenna from a direct and a reflected or diffracted path after being reflected or diffracted by an object (El-Rabbany, 2006). The path travelled by the reflected signal is always longer than the direct path. At the satellite, multipath can be attributed to reflections, which are called satellite multipath (Hofmann-Wellenhof et al., 2008). Multipath is more severe at the receiver than the satellite and it is also more significant on the code measurements than on the carrier-phase measurements. Multipath is not like other GPS errors that can easily modelled or removed by differential positioning. Multipath is site-dependent, so modelling it is a very complicated task. However, there are various techniques and methodologies recommended for multipath mitigation, including careful site selection, using special antenna types (such as a choke-ring), and using receivers that employ mitigation algorithms at the receiver signal processing level (Elsobeiey, 2012). The Multipath Elimination Delay lock loop (MEDLL) is an example of a mitigation algorithm. However, antenna and receiver mitigation techniques are not significant in mitigating multipath caused by reflectors located within 30 m of the GPS antenna (Zhong et al., 2007).

For kinematic mode positioning like in the case of land vehicle navigation, modelling multipath is a very complicated task. In this research, TEQC software (TEQC, 2016) was used to estimate multipath. The algorithm in TEQC is GNSS pre-processing software developed by University of Colorado (UNAVCO). Teqc uses the linear combination of L1 and L2 to obtain multipath on L1 and L2, as in the following (Louis and Charles, 1999):

$$MP1 = P_1 - \left(1 + \frac{2}{\left(\frac{f_1^2}{f_2^2} - 1\right)}\right) L_1 + \left(\frac{2}{\left(\frac{f_1^2}{f_2^2} - 1\right)}\right) L_2 \quad (2.20)$$

$$MP2 = P_2 - \left(1 + \frac{2 * \left(\frac{f_1^2}{f_2^2}\right)}{\left(\frac{f_1^2}{f_2^2} - 1\right)}\right) L_1 + \left(\frac{2 * \left(\frac{f_1^2}{f_2^2}\right)}{\left(\frac{f_1^2}{f_2^2} - 1\right)}\right) L_2 \quad (2.21)$$

Here *MP1* and *MP2* are the multipath on *L1* and *L2* linear combinations. *P1* and *P2* are the pseudorange measurements on *L1* and *L2*, L1 and L2 are the carrier phase measurements on *L1*, and *L2* and f_1 and f_2 are the frequencies of *L1* and *L2*.

2.11.8 Satellite hardware delay estimation

According to El-Rabbany (2006), hardware delay is the time delay that happens in the satellite and the receiver. Satellite hardware delay is the difference between the time a signal is generated and the time it is transmitted by the satellite antenna. Receiver hardware delay is the difference between the time a signal is received by the receiver antenna and the time the signal is processed inside the receiver correlator. Hardware delays are frequency-dependent — this is different for L1 and L2 carriers. Because hardware delays in the satellite and the receiver are different, they are also different for the code and the carrier, and also between L1 and L2 carriers. The differences between the delays on L_1 and L_2 are called “inter-frequency” or “differential bias.” Hardware delay can be calibrated using GPS measurements collected at reference stations.

Undifferenced code and carrier phase dual frequency observations can be written as:

$$C_1 = \rho + dT - dt + I_1 + \lambda_1 b_{C1}^r - \lambda_1 b_{C1}^s + \varepsilon_{C_1} \tag{2.22}$$

$$P_1 = \rho + dT - dt + I_1 + \lambda_1 b_{P1}^r - \lambda_1 b_{P1}^s + \varepsilon_{P_1} \tag{2.23}$$

$$P_2 = \rho + dT - dt + \frac{f_1^2}{f_2^2} I_1 + \lambda_2 b_{P_2}^r - \lambda_1 b_{P_2}^s + \varepsilon_{P_2} \tag{2.24}$$

$$\Phi_1 = \rho + dT - dt - I_1 + \lambda_2 b_{\Phi_1}^r - \lambda_2 b_{\Phi_1}^s + \lambda_1 N_1 + \varepsilon_{\Phi_1} \tag{2.25}$$

$$\Phi_2 = \rho + dT - dt - \frac{f_1^2}{f_2^2} I_1 + \lambda_2 b_{\Phi_2}^r - \lambda_1 b_{\Phi_2}^s + \lambda_2 N_2 + \varepsilon_{\Phi_2} \tag{2.26}$$

Here C_i is the measured C/A-code pseudorange on L_i (m); P_i is the measured P-code pseudorange on L_i (m); ρ is the true geometric range (m); dT is the receiver clock error (m); dt is the satellite clock error; I_1 is the ionospheric delay on L_i (m); f_i is the frequency on L_i (Hz); N_i is the integer ambiguity term in measured carrier phase on L_i (cycle); b_*^r is the receiver hardware delay bias on measurement type * (cycle); b_*^s is the satellite hardware delay bias on measurement type * (cycle); and ε_* is noise, including residual multipath (m).

When one receiver is used, the receiver hardware delay is the same for different satellites, which means satellite-satellite single difference (SSSD) at the receiver could completely remove the receiver hardware delays.

If a network of receivers is used, a certain satellite's hardware delay is the same for all receivers that observe this satellite. In this case, a receiver-receiver single difference (RRSD) could completely remove this satellite hardware delay.

2.11.9 Satellite and receiver initial phase bias

According to Wang and Gao (2007b), for dual-frequency GPS data, the undifferenced C/A code and phase observation equations can be written as:

$$C_1 = \rho + c(dT - dt) + d_{orb} + d_{trop} + d_{ION/L_1} + \varepsilon(C_1) \quad (2.27)$$

$$\Phi_1 = \rho + c(dT - dt) + d_{orb} + d_{trop} + d_{ION/L_1} + \lambda_i[N_i + \phi_r(t_0, L_i) - \phi_s(t_0, L_i)] + \varepsilon(\Phi_i) \quad (2.28)$$

here i = 1, 2 and C_1 is the measured C/A pseudorange on L_1 (m); Φ_i is the measured Li carrier phase (m); ρ is the true geometric range (m); c is the speed of light (m/s); dt is the satellite clock error (s); dT is the receiver clock error (s); d_{trop} is the troposheric delay m; d_{orb} is the satellite orbit error; $d_{ion/Li}$ is the ionospheric delay (m); λ_i is the wavelength on Li (m/cycle); Ni is integer phase ambiguity on L_i (cycle); $\phi_r(t_0, L_i)$ is the initial phase bias of receiver oscillator on Li (cycle); $\phi_s(t_0, L_i)$ is the initial phase bias of the satellite oscillator on Li (cycle); $\varepsilon(\Phi_i)$ is the noise including residual multipath (m).

The wide-lane (WL) and geometry-free (GF) combinations can be written as:

$$\Phi_{WL} = \frac{f_1\Phi_1 - f_2\Phi_2}{f_1 - f_2} \tag{2.29}$$

$$\begin{aligned}\Phi_{WL} = \rho + c(dT - dt) + d_{orb} + d_{ION/L_1} + d_{trop} \\ + \lambda_{WL}[N_1 + \phi_r(t_0, L_1) - \phi_s(t_0, L_1)] \\ - \lambda_{WL}[N_2 + \phi_r(t_0, L_1) - \phi_s(t_0, L_1)] + \varepsilon(\Phi_{WL})\end{aligned} \tag{2.30}$$

$$\Phi_{GF} = \Phi_1 - \Phi_2 \tag{2.31}$$

$$\begin{aligned}\Phi_{GF} = \frac{f_1^2 - f_2^2}{f_2^2} d_{ION/L_1} + \lambda_1[N_1 + \phi_r(t_0, L_1) - \phi_s(t_0, L_1)] \\ - \lambda_2[N_2 + \phi_r(t_0, L_2) - \phi_s(t_0, L_2)] + \varepsilon(\Phi_{GF})\end{aligned} \tag{2.32}$$

2.11.9.1 Receiver initial phase bias determination

Zero-baseline RRSD observations can be used to eliminate all satellite related errors, including the satellite initial phase bias, and to estimate the receiver initial phase bias.

$$\Delta C_1 = c\Delta dT + \varepsilon(\Delta C_1) \tag{2.33}$$

$$\Delta\Phi_{WL} = c\Delta dT + \lambda_{WL}[\Delta N_1 + \Delta\phi_r(t_0, L_1)] - \lambda_{WL}[\Delta N_2 + \Delta\phi_r(t_0, L_2)] + \varepsilon(\Delta\Phi_{WL}) \tag{2.34}$$

$$\Delta\Phi_{GF} = \lambda_{GF}[\Delta N_1 + \Delta\phi_r(t_0, L_1)] - \lambda_{GF}[\Delta N_2 + \Delta\phi_r(t_0, L_2)] + \varepsilon(\Delta\Phi_{GF}) \tag{2.35}$$

Differencing between RRSD C/A code and WL carrier phase combinations will remove RRSD receiver clock errors.

$$\Delta\tilde{N}_{WL} = \frac{\Delta\Phi_{WL} - \Delta C_1}{\lambda_{WL}} = [\Delta N_1 + \Delta\phi_r(t_0, L_1)] - [\Delta N_2 + \Delta\phi_r(t_0, L_2)] \tag{2.36}$$

RRSD integer ambiguity can be estimated using the calculated hybrid RRSD WL ambiguity and GF observations:

$$\begin{bmatrix} \Delta N_1 \\ \Delta N_2 \end{bmatrix} = round\left\{ \begin{bmatrix} 1 & -1 \\ \lambda_1 & -\lambda_2 \end{bmatrix}^{-1} \begin{bmatrix} \Delta\tilde{N}_{WL} \\ \Delta\Phi_{GF} \end{bmatrix} \right\} \tag{2.37}$$

The estimated RRSD integer ambiguity from the above equation will be used to determine the receiver initial phase bias while its correctness will be validated according to the following procedures:

1. Calculate a derived DD ambiguity.

$$\nabla\Delta N_i^j - \nabla\Delta N_i^k = (\Delta N_i^j - \Delta N_i^{ref}) - (\Delta N_i^k - \Delta N_i^{ref}) = \Delta N_i^j - \Delta N_i^k \tag{2.38}$$

2. Obtain a calculated Double Difference (DD) ambiguity.

$$\nabla\Delta\Phi_i = \lambda_i \nabla\Delta N_i + \varepsilon(\nabla\Delta\Phi_i) \tag{2.39}$$

$$\nabla\Delta N_i = round\left(\frac{\nabla\Delta\Phi_i}{\lambda_i} \right) \tag{2.40}$$

RRSD inter-frequency receiver initial phase bias can be written as:

$$\lambda_1\Delta\phi_r(t_0, L_1) - \lambda_2\Delta\phi_r(t_0, L_2) = \Delta\Phi_{GF} - (\lambda_1 N_1 - \lambda_2 N_2) \tag{2.41}$$

2.11.10 Periodic relativistic

According to Conley et al. (2006), the relativistic periodic effect is a relativistic error that arises because of the slight eccentricity of satellite orbits. Orbit eccentricity causes changes in satellite speed and in the gravitational potential experienced by a satellite. These two changes contribute equally to periodic relativistic error. Exactly 50% of the periodic effect is caused by the periodic change in the speed of a satellite relative to the ECI frame; the other 50% is caused by the satellite's periodic change in its gravitational potential. The satellite experiences higher velocity and lower gravitational potential at perigee — and both cause the satellite's clock to run more slowly. The satellite experiences lower velocity and higher gravitational potential at apogee — and both cause the satellite's clock to run faster (Hatch, 1995; Ashby and Spilker, 1996). This effect can be compensated for in the following way (ARINC Research Corporation, 2004):

$$\Delta t_r = Fe\sqrt{a}\sin E_k \tag{2.42}$$

where F is $-4.442807633 \times 10^{-10}$ s/m$^{1/2}$; e is satellite orbital eccentricity; a is the semi major axis of the satellite orbit; and E_k is the eccentric anomaly of the satellite orbit Sagnac.

2.11.11 Sagnac Effect

According to Conley et al. (2006), the Sagnac effect is a relativistic error in the range between the satellite and the receiver. It occurs because of the rotation of the Earth during the time of signal transmission, when satellite coordinates are computed in an ECEF coordinate system. During the signal propagation time, if the receiver experiences a net rotation away from the satellite, the propagation time will increase, and vice versa (see Figure 2.1). The Sagnac effect can introduce

positioning errors on the order of 30 m (Ashby and Weiss, 1999). Corrections for the Sagnac effect are often referred to as Earth rotation corrections and can be computed as follows:

$$\Delta t_{Sagnac} = \frac{\vec{r_r^s}.\vec{v_r^s}}{c} \tag{2.43}$$

Here $\vec{r_r^s}$ is the instantaneous position vector between the receiver and the satellite, $\vec{v_r^s}$ is the instantaneous velocity vector between the receiver and the satellite, c is speed of light, and is the dot-product operator.

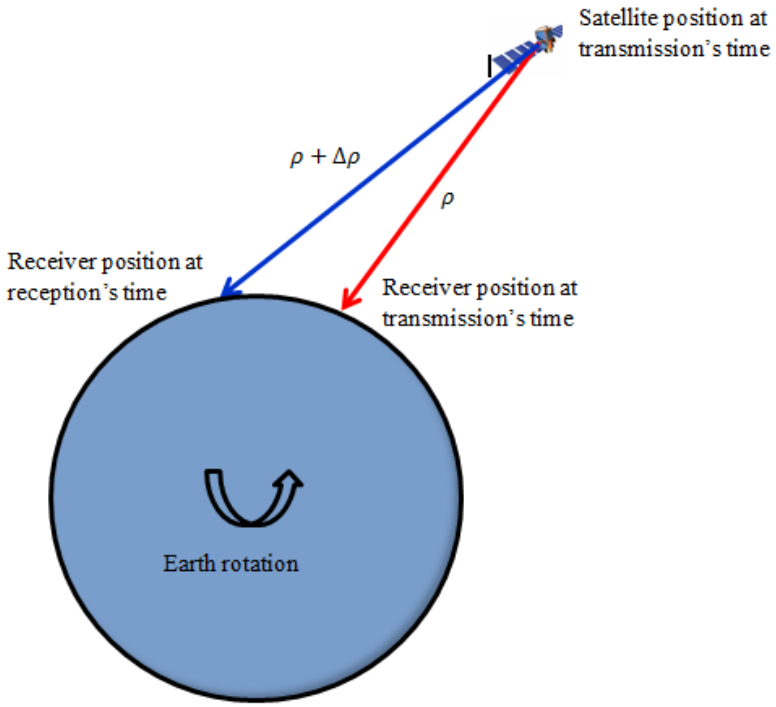

Figure 2.1 Sagnac Effect

2.11.12 Satellite Geometry

Satellite geometry is the geometric locations of GPS satellites with respect to the receiver's antenna the. Satellite geometry is described as good when the tracked satellites are spread out in the sky (El-Rabbany, 2006). A dilution of Precision (DOP) is a quantity that can be used to identify the quality of satellite geometry instantaneously (Hofmann-Wellenhof *et al.*, 2008). Small DOP values are indicators to good satellite geometry and vice versa. The effect of satellite geometry on the three dimensional antenna's position (latitude, longitude, and height) is known as Position Dilution of Precision (PDOP). PDOP can be separated in into two parts: Horizontal DOP (HDOP) which represents the geometry effect on the horizontal 2D position, and Vertical DOP (VDOP) which represents the effect on the vertical position component. Another variant of DOP is Geometric DOP (GDOP). Generally, DOP variants can be computed from the 3D position and receiver's clock error variances PDOP and GDOP can be computed using the solution covariance matrix transformed to the local coordinates frame as:

$$PDOP = \sqrt{C_{xx} + C_{yy} + C_{zz}} \tag{2.44}$$

$$GDOP = \sqrt{C_{xx} + C_{yy} + C_{zz} + C_{tt}} \tag{2.45}$$

where C_{xx}, C_{yy} , and C_{zz} are the variances of x-coordinate, y-coordinate, and z-coordinate of the receiver's antenna position respectively, and C_{tt} is receiver's clock error

Chapter 3

3 Inertial Navigation System

An Inertial Navigation System (INS) is a navigation aid that measures acceleration and rotation rate to determine the navigation state of a moving object without the need for external references. It is used on vehicles such as ships, aircraft, submarines, guided missiles, and spacecraft. A typical INS is composed of an accelerometer and a gyroscope as well other electronics and a computer. Accelerometers sense and record acceleration and gyroscopes sense and measure angular rate changes. Acceleration and angular rate change measurements are used by INS to provide information about position, velocity, and attitude of a moving platform using the principle of Dead Reckoning (DR). DR is the process of generating the current position of a moving platform using knowledge of its previous position and inertial sensors measurements. Inertial sensors measure accelerations and angular rotations. Given initial conditions, acceleration is integrated to provide velocity and a second integration produces position. Angular rates are processed to give the attitude (pitch, roll, and yaw angles) of the moving platform and also to enable navigation parameter transformation from body frame to Local Level Frame (LLF). The accelerometer and gyroscopes constitute an Inertial Sensor Assembly (ISA), which output the raw data from the sensors. ISA and other related electronics are housed in a unit called the Inertial Measurement Unit (IMU). The output from the IMU is the sensors raw data corrected from errors such as scale factors and biases.

Any INS fall in one of two categories: (1) a grimballed system or stable system; (2) a strapdown system. In a grimballed system, inertial sensors are mounted in such a way that they are always aligned with the navigation frame. This system allows direct integration of the accelerations to obtain position right away in the navigation frame. The drawback of grimballed systems is that they are expensive and complex.

Inertial sensors rigidly mounted to the body of the moving platform characterize strapdown systems. Unlike griballed systems, which mechanically transform between body and navigation frames, strapdown systems use processors that implement software's frame transformation. Therefore, the gyroscope measurement continuously updates the transformation between the body and navigation frames. This transformation is then applied to the accelerometer measurements to project the accelerations to the navigation frame. Strapdown systems have the advantage of being reliable, flexible, and light. They also consume low amounts of power, and are inexpensive when compared with gimballed systems. Table 4.1 highlights comparisons between grimballed and strapdown systems.

Table 4.1 Comparison Between Gimballed and Strapdown Systems

	Size	**Weight**	**Performance**	**Robustness**
Grimballed System	Big	Light	Superior accuracy	High reliability and low immunity to shock and vibration
Strapdown System	Small	Heavy	High accuracy	High reliability and immunity to shock and vibration

3.1 INS Principle of Operation

An INS is composed of an IMU and a navigation processor. An IMU includes a mutually orthogonal triad of accelerometers and a mutually orthogonal triad of gyroscopes. Nominally, the axes of both triads are parallel and the origin is defined as the origin of the accelerometer triad (Karamat, 2009; Georgy, 2010). In strapdown systems, the sensor axes are mounted inside the body of the IMU and are called the body axes. The navigation processor includes two modules: (1) a pre-processing module; (2) a mechanization module (Noureldin, 2007; Georgy, 2010). The pre-processing module is to filter out any unwanted disturbances in the signals produced by the IMU. The mechanization module processes these filtered signals and transforms them into position and attitude parameters. Figure 4.1 illustrates the three major modules of an INS.

3.2 Inertial Sensors

Inertial sensors are position, attitude, or motion sensors whose references are completely internal except possibly for initialization (Aggarwal, 2010). Two sensors re included in an IMU: the accelerometer and the gyroscope.

3.2.1 Accelerometer

An accelerometer is an inertial sensor that measures the linear acceleration of a body along the body sensitive axis (Aggarwal, 2010). Accelerometers measure the specific force in an inertial reference frame, which can be used to estimate the acceleration of the moving body. Therefore, the accelerometer in the direction of motion of a vehicle may be most important as it should contain

all the information. For three-dimensional motion, however, it is best to record the accelerations in all three directions to accurately model the motion.

3.2.2 Gyroscope

A gyroscope is an inertial sensor that measures the rate of rotation in an inertial fame about its sensitive axis (Aggarwal, 2010).

3.2.3 Inertial Sensors Classification

Many parameters can be used in inertial sensor classifications, such as accuracy, price, quality of components, applications, and so on. In the literature, there is no certain or one classification of inertial sensors. In general, the specifications of the gyroscope play a big role in determining the quality of an IMU. The gyroscope's quality depends mainly on the gyroscope's bias, which is given in the unit of deg per hr, and also on the gyroscope random walk, which is normally expressed in the unit of deg per square-root hr. Karamat (2009) and Georgy (2010) compare different inertial sensors and systems and their research is summarized in Table 4.2.

Table 4.2 Classification of Inertial Navigation Systems, (Georgy, 2010)

Performance	**Strategic Grade**	**Navigation Grade**	**Tactical Grade**	**Commercial Grade**
Position Error	30 – 100 m/hr	1 nm/hr or 0.5 m/sec	10 – 20 nm/hr	Large variation
Gyroscope Drift	0.0001 – 0.001 $\frac{deg}{hr}$	< 0.01 $\frac{deg}{hr}$	1 – 10 $\frac{deg}{hr}$	0.1 – 1 $\frac{deg}{sec}$
Gyroscope Random Walk		< 0.002 $deg/\sqrt{hr}$	0.05 – 0.2 $deg/\sqrt{hr}$	Several $deg/\sqrt{hr}$
Accelerometer Bias	0.1 – 1	< 100 mg	1 – 5 mg	10 – 1000 mg
Approximate Cost	➢ $100,000	$50,000 – $100,000	$5,000 – $20,000	$200 – $2000
Application	Submarines, intercontinental ballistic missiles	General navigation, high precision, geo-referencing, mapping	Integrated with GPS/GNSS for mapping, weapons (short time)	Research, low-cost navigation, pedometers, antilock braking, active suspension, airbags

3.3 Microelectromechanical Systems

Microelectromechanical Systems (MEMS) are the integration of mechanical elements, sensors, actuators, and electronics on a common substrate. MEMS bring together silicon-based microelectronics and micromachining technologies on a common platform.

3.4 Inertial Sensors Errors

According to Noureldin (2009), all INS sensors are subject to errors that limit the accuracy with which an observable can be measured. MEMS sensors are more affected with errors compared with mechanical sensors (Aggarwal, 2010). The performance characteristics of inertial sensors (either accelerometers or gyroscopes) are described in terms of sensor bias, sensor scale factor, and sensor noise. Figure 4.1 show the different errors that affect INS sensor measurements.

Bias generally consists of two parts: the first part is deterministic and is called “bias offset,” which refers to the offset in measurements provided by the inertial sensor. The second part is stochastic and is called “bias drift,” which refers to the rate at which the error in an inertial sensor accumulates over time. The bias offset is deterministic in nature and can be determined by a sensor’s calibration. The bias drift is random in nature and should be modeled stochastically.

Scale factor is the relationship between the output signal of the sensor and the physical quantity being measured. Scale factor errors are deterministic and can be obtained by a sensor’s calibration. Scale factor stability is the capability of the inertial sensor to accurately sense angular velocity or acceleration at different angular rates or accelerations, and is presumed to mean the variation of scale factor with temperature, as well as its repeatability.

Sensor noise is an additional signal that interferes with the inertial sensor output. Noise comes from the sensor itself or other electronic equipment. Noise in general is non-systematic and therefore cannot be removed from the measurements using deterministic models. It can only be modeled stochastically.

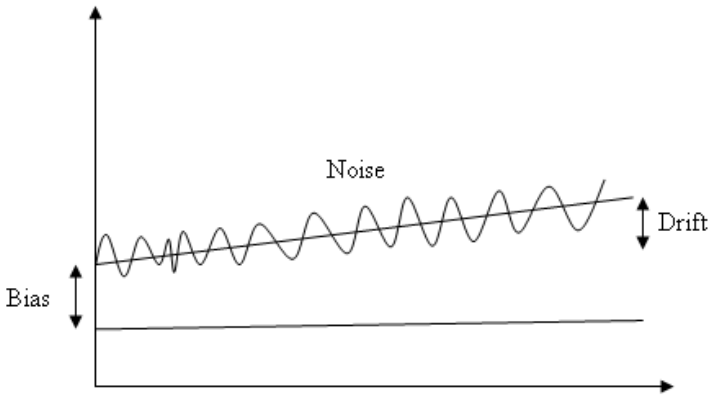

Figure 4.1 Sensor Bias Offset, Bias Drift and Noise

3.5 Inertial Sensors Systems

3.6 Odometry Data

Land vehicles or any moving objects that are equipped with motion sensors can provide useful data that can be used to obtain navigational information. The process of converting this data into navigational information is called odometry. The idea behind the odometer is that it integrates incremental motion information over time. In land vehicles, odometry data is extracted using sensors that measure the rotation of the vehicle wheel axes and the steer axes. Wheel rotation is translated into curvilinear distance. A disadvantage of odometry is that it is sensitive to errors because of the integration of velocity measurements over time to produce position estimates. In addition, orientation errors cause large errors in positioning that increase with the distance

travelled by the vehicle. A major cause of error in this process is wheel slippage. On the other hand, the advantages of odometry data are that they have short-term accuracy, are inexpensive, and allow high sampling rates. The integration of incremental motion information over time leads to another disadvantage: any small constant error increases after integration. Furthermore, orientation errors cause large position errors that increase with the distance travelled by the vehicle. To ensure effective use of odometry data, accurate data collection, equipment calibration, and processing are required.

3.7 Reduced Inertial Sensors System

The Reduced Inertial Sensors System (RISS) is a low-cost inertial sensor system that uses one gyroscope to sense rotation rate about the body frame vertical axis (Z-axis) and two accelerometers to sense acceleration along the body frame two horizontal axes (X- and Y- axis). The reasons for using three sensors instead of six is to reduce random errors and noise associated with low-cost MEMS and to cut the system cost. RISS compensates for the vertical accelerometer and the two horizontal gyroscopes by using the moving platform odometry data. In RISS, the relationship between the measurements and the desired navigation information can be summarized as in Figure 4.2 and highlighted as follows:

The gyroscope's measurement is used to compute the azimuth.

- The accelerometers' measurements, along with the vehicle speed, obtained from the odometry data, and reliable for the Earth gravity, are used to compute other attitude angles' pitch and roll.

- The forward speed, obtained from the odometry data, along with the computed azimuth and pitch angles are used to compute the vehicle's local-level frame velocities (east, north, and up velocities).

The computed east and north velocities, along with ellipsoid parameters, are used to compute the three-dimensional position components (latitude, longitude, and height).

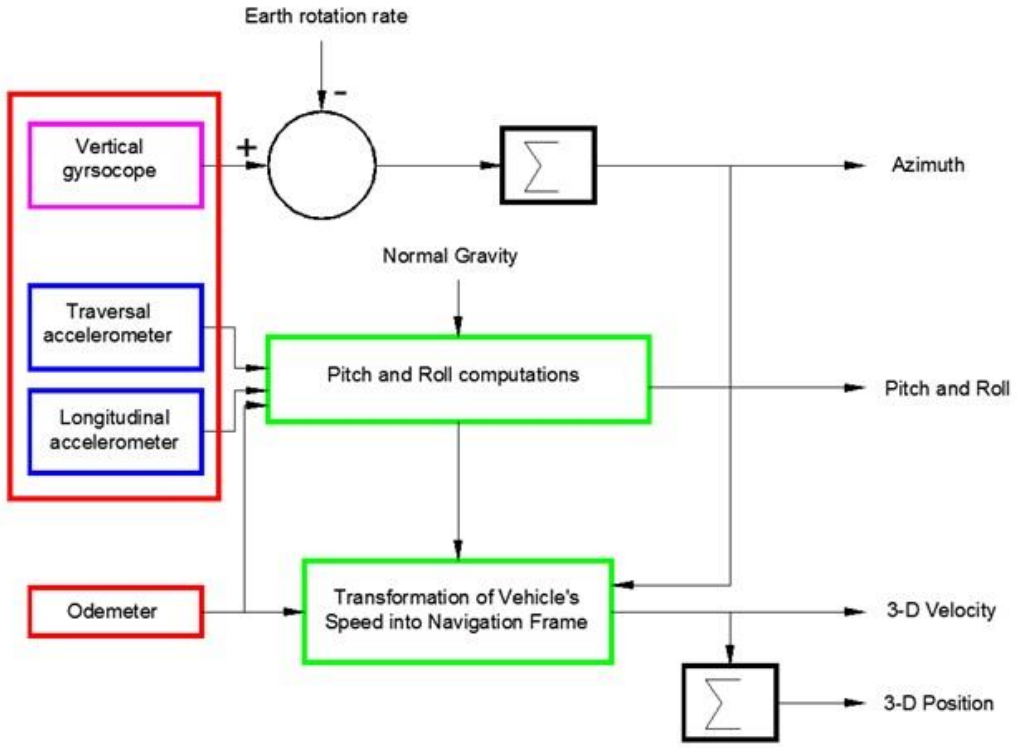

Figure 4.2 RISS Flow Chart (Karamat, 2014)

3.8 RISS Motion Equation

To ensure discussion and derivations, the LLF is based on east, north and up (ENU) directions and the X, Y, X axes of the body are aligned with traversal, forward, and vertically upward directions, respectively. The three-dimensional RISS navigation state vector is defined as x = $[\varphi,\lambda,h,v_e,v_n,v_u,r,p,A]^T$, where φ is latitude, λ is the longitude, h is altitude, v_e is east velocity, v_n is north velocity, v_u is upward velocity, r is the roll, p is the pitch, and A is the azimuth (Karamat, 2014).

Azimuth

To obtain the azimuth (A), the gyroscope reading w_z is -integrated. Before integrating w_z, three corrections must be applied in order to get the actual azimuth rate of change. The first correction is for the gyroscope bias (b_z). This bias must be subtracted from the gyroscope measurements. Then the unbiased gyroscope measurement is corrected for the rotation of the Earth and moving of the origin of the LLF with respect to the centre of the Earth. The azimuth rate of change in the LLF can be written as:

$$\dot{A} = -\left[(w_z - b_z) - w^e \cos\varphi - \left[\frac{v^e \tan\varphi}{R_N - h}\right]\right] \tag{4.1}$$

where w_z is the vertical gyroscope's measurement, b_z is the estimated gyroscope bias, w_e is the Earth's rotation rate, h is the altitude of the vehicle, and R_N is the normal radius of curvature of the Earth's ellipsoid. In equation (4.1) above the gyroscope's measurement w_z is being compensated for the bias b_z. This also affects its linearization, as we will see in chapter 5 of this thesis.

Pitch

The idea behind the computation of pitch and roll angles is presented in Noureldin et al, (2002) and Noureldin et al, (2004). When a vehicle is moving, the vehicle's forward acceleration as well as the component of gravity due to pitch angle are measured by the longitudinal accelerometer. Hence, in computation of the pitch angle *p*, the vehicle acceleration (a_0) derived from the vehicle's speed measurement is subtracted from the forward accelerometer measurement (f_y). Mathematically, the computation of the pitch angle is expressed as:

$$p = \sin^{-1}\left(\frac{f_y - a_0}{g}\right) \tag{4.2}$$

where *g* is the Earth's normal gravity.

Roll

Lateral component of vehicle acceleration during a vehicle's turn and the component of gravity as result of the roll angle are measured by the transversal accelerometer. Therefore, the computation of the roll angle *r*, requires compensation to the traversal accelerometer measurement (f_x) by the lateral component of acceleration. The computation of the roll angle is therefore given as:

$$r = -\sin^{-1}\left[\frac{f_x + v_0(w_z - b_z)}{g\cos p}\right] \tag{4.3}$$

in equation (4.3) v_0 is the vehicle speed obtained from the wheel rotation sensor measurement, w_z is the angular rate measured by the vertically aligned gyroscope, and $v_0 w_z$ is the lateral component of acceleration experienced during a turn.

Velocities

The computation of the vehicle velocities along east, north and up velocities (v_e, v_n, and v_u, respectively) is a process of transformation of a vehicle's speed along a forward direction v_0 along east, north and up axes. The transformation require the knowledge of azimuth and pitch angles and done according to the following expressions:

$$v_e = v_0 \sin A \cos p \tag{4.4}$$

$$v_n = v_0 \cos A \cos p \tag{4.5}$$

$$v_u = v_0 \sin p \tag{4.1}$$

Position

Position is the product of the integration of velocity, so to obtain position in geodetic coordinates we must have the velocity in the geodetic coordinate system. In the case here, we have the velocities in LLF (East, North and U) and we need the position as geodetic coordinates. Then we must first transfer Eat and North velocities into geodetic coordinates and then integrated over the sample interval to obtain positions in latitude and longitude. The altitude h is obtained by directly integrate the Up velocity. The three position quantities are calculated using the following expressions:

$$\dot{\varphi} = \frac{v_n}{(R_M + h)} \tag{4.2}$$

$$\dot{\lambda} = \frac{v_e}{(R_N + h)\cos\varphi} \tag{4.8}$$

$$\dot{h} = v_u \tag{4.9}$$

where R_M is the meridian radius of curvature and R_N is the normal radius of curvature of the Earth's ellipsoid. A block diagram that illustrates RISS mechanization system is shown in Figure 4.3. A close look at equations 4.1 through 4.6 reveals that the major source of errors in the RISS system is errors in gyroscope measurements (gyroscope bias b_z) because these measurements prorogate to azimuth, which in turn prorogates into an error in horizontal channel velocity and position. Since no integration is involved in roll and pitch calculation in equations 4.2 and 4.3, errors in accelerometer measurements have only a small effect and can be reduced using common sensor calibration techniques.

Chapter 4

4 GPS/INS Integration

GPS/INS integration is a form of sensor integration or fusion that involves combining the output or observations of different sensor systems to obtain a better estimation of their navigation state (Grewal et al., 2007).

4.1 Advantages of GPS/INS Integration

According to Grewal et al. (2007), to benefit from the opposite characteristics of GPS and INS, INS and GPS are integrated to obtain good positioning solutions. In the short term, INS produces accurate navigation parameters; however, its errors become greater in the long term because of intrinsic sensor errors. On the other hand, GPS's accuracy is more consistent in the long term and does not degrade over time. However, for short-term use, GPS is not as accurate as INS. In addition, GPS signals may suffer from outages due to signal blockages, interference, or jamming, while INS is immune to these effects. GPS outages may happen, for example, in urban canyons or tunnels. Thus, to have a continuous positioning solution with higher performance, INS and GPS are integrated. More advantages of INS/GPS integration are as follows:

- INS enables continuous positioning by providing solutions during GPS outages.
- INS provides navigation solutions at a higher rate than that provided by GPS.
- INS can determine the full navigation state without differentiation, as with GPS. This has an advantage of low sensitivity to high noise.

- Integrating GPS with INS reduces the effect of GPS errors.
- GPS aid is used for online calibration of IMU errors and online alignment of the IMU, as well as estimation and correction of errors in the INS state. Furthermore, GPS can be used to compensate for the long-term drift of INS readings. As a result, during GPS outages, INS error growth is less due to online calibration and alignment.
- INS can supply accurate position and velocity information during short periods and subsequently reduce the PPP solution convergence time.

4.2 GPS/INS Integration Schemes

The GPS/INS integration scheme can simply be categorized as loosely coupled and tightly coupled (Grewal et al., 2007), and ultra-tightly coupled. In the loosely coupled integration (Figure 5.1), GPS and INS work as two independent navigators and the integrated system between them as a third navigator (Gebre-Egziabher, 2007). In this integration architecture, the derived position and velocity estimates of the GPS receiver are used to update the INS position and velocity estimates. KF utilizes a linearized model of INS errors and provides estimates of these errors based on the GPS updates. Therefore, in loosely coupled GPS/INS integration, GPS navigation solutions are essential. No GPS/INS solution can be obtained when less than four satellites are tracked.

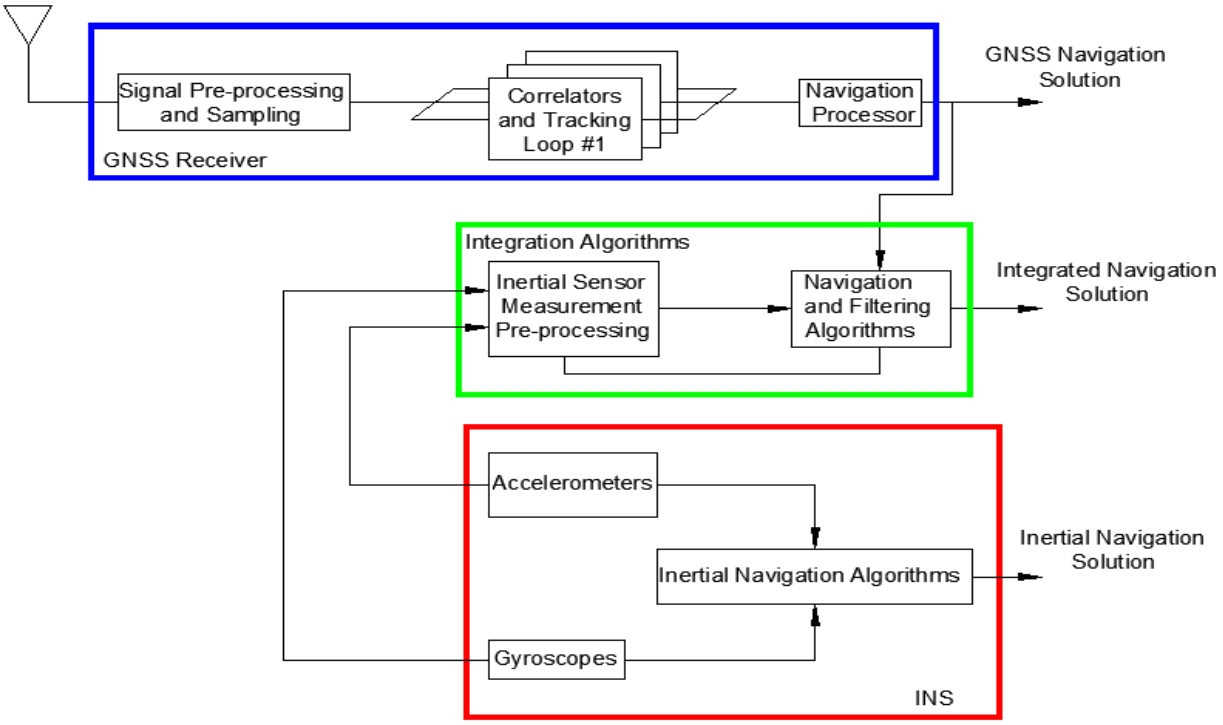

Figure 4.1 GPS/INS Loosely-Coupled Integration (Gebre-Egziabher, 2007)

In the tightly coupled scheme (Figure 5.2), the GPS pseudorange, carrier phase, and pseudorange's rate of change (Doppler measurements) are used as observations to update the navigation filter. The INS estimated solution can continuously be updated by the GPS measurements, even if the number of tracked GPS satellites drops below four.

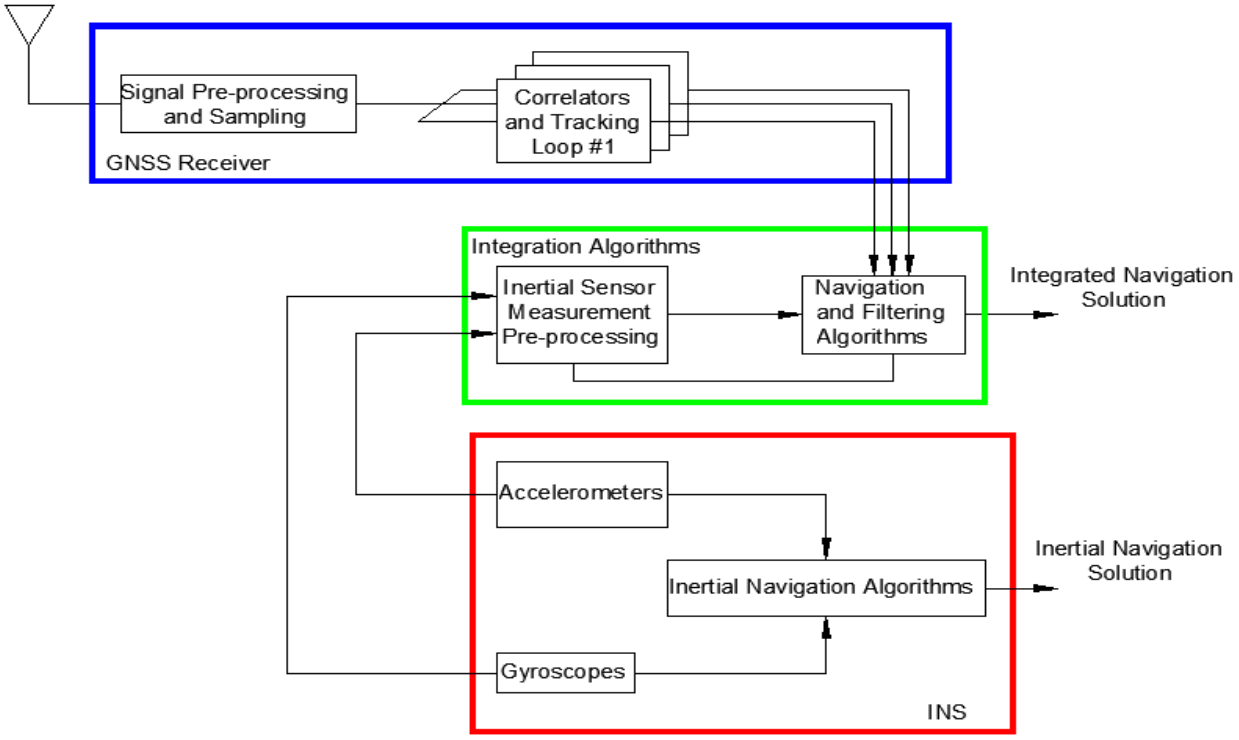

Figure 4.2 GPS/INS Tightly-Coupled Integration (Gebre-Egziabher, 2007)

Ultra-Tightly (UT) coupled integration is distinguished from loose and tight architecture integration by two features. Firstly, there is a fundamental difference in the architecture of the GNSS receiver: in the first two integration schemes, the receiver architecture consists of a bank of independent code and carrier-phase tracking loops, whereas in the ultra-tight scheme, the receiver architecture consists of a single Vector Delay Lock Loop (VDLL). Secondly, the use of INS data in the ultra-tight scheme is an integral part of the GNSS receiver, so the independence of GNSS and INS as navigators disappears. However, there is an advantage to ultra-tight integration — the ultra-tight scheme improves the robustness of the GNSS to signal jamming and interference. Ultra-

tight integration involves a complex process of INS/GNSS data fusion inside the GNSS receiver. However, if this complexity resolved, ultra-tight architecture is considered an optimal INS/GNSS integration (Gebre-Egziabher, 2007).

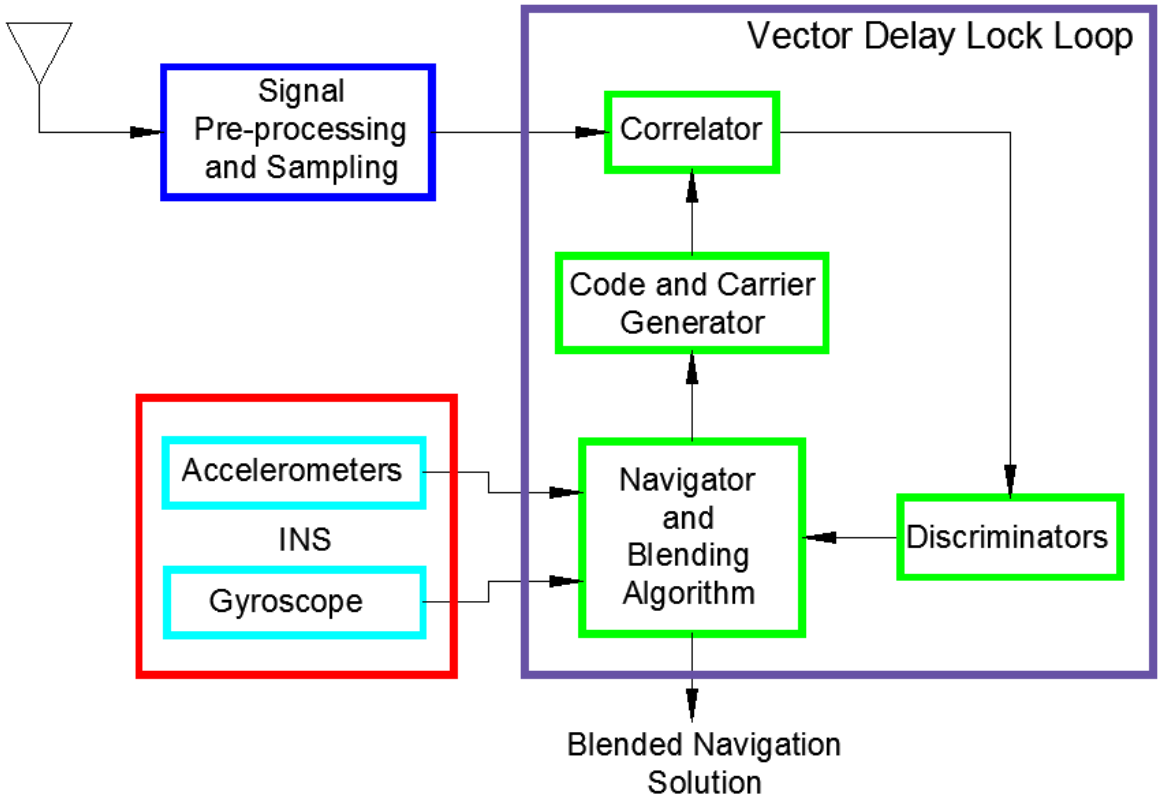

Figure 4.3 GPS/INS Ultra-Tightly Coupled Integration (Gebre-Egziabher, 2007)

4.3 GPS/INS Integration Estimation Techniques

The purpose behind GPS/INS integration is the use of the complementary characteristics of the two systems in a mutual relation. The optimal benefit of this relation is more superior accuracy than any of the systems can offer when used alone. This accuracy also depends on the estimation's algorithm used. The primary algorithm for GPS/INS is KF: however, KF has limited capability in handling non-linear errors, which is the case with GPS and INS errors.

4.3.1 Kalman Filter

The Kalman filter (KF) is an optimal estimation tool that provides a sequential recursive algorithm to solve linear systems (Minkler and Minkler, 1993). KF's main advantage as an optimal estimator is its suitability for applications of quantitative error analysis that require real-time statistical information and estimation accuracy of the system states (Grewal and Andrews, 2001). KF produces its own error analysis as a projection of the error covariance matrix, which gives an indication of the estimation accuracy.

In estimating system states, KF starts with a suitable approximation of the state and then follows a procedure that implements two groups of equations: prediction equations and measurement update or correction equations (Georgy, 2012). The time-update equations are used to predict forward in time current state and error covariance estimates to obtain the *a priori* estimates for the next step. Measurement update equations are used to include new measurement into the *a priori* estimate to obtain an improved *a posteriori* estimate (Welch and Bishop, 2001).

According to (Ristic et al., 2004), in most practical situations optimal non-linear filters cannot be applied. Instead, suboptimal solutions are utilized. A number of approximate or suboptimal filters have been proposed. These proposed filters can be grouped into four broad

classes: (1) analytic approximations; (2) numerical approximations; (3) Gaussian sum or multiple model filters; (4) sampling approaches. The sampling approach is used to approximate the posterior density using a set of samples. The unscented Kalman filter (UKF) and the PF are filters of the last category. The UKF uses a small number of deterministically chosen samples, while the PF uses a large number of random samples.

In order to accurately model the underlying dynamics of a physical system, it is important to include elements of non-linearity and non-Gaussian sums in many application areas. Particle filters (PF) can be used to achieve this. PF is a sequential Monte Carlo method based on point-mass representations of probability densities, which are applied to any state model. PF is concerned with the problem of tracking single and multiple objects. It is a hypothesis tracker that approximates the filtered posterior distribution using a set of weighted particles. It weights particles based on a likelihood score and then propagates these particles according to a motion model. The disadvantages of PFs include high computational complexity, difficulty in determining optimal number of particles, increases in number of particles with increasing model dimensions, as well as other potential problems, including degeneracy and loss of diversity.

4.3.2 Extended Kalman Filter (EKF)

EKF is a variant of KF, so in this regard discussing its background requires a review of KF's formation. There are a variety of resources in the literature that offer details about KF and its variants. Of these excellent resources, we refer to studies and books by Brown and Hwang (1997), Grewal and Andrews (2008), Simon (2006), and Gelb (1976).

EKF is considered a suboptimal estimation technique that overcomes PF drawbacks. EKFs operate in the framework of Gaussian approximation for posterior density. While this gives them

the advantage to be simply implemented and executed, it also suffers from an inherent inability to model higher order moments of non-Gaussian posterior densities (Ristic et al., 2004).

In the following, EKF is summarized, for which KF is based on the following system and measurement models. The system model can be represented as follows:

$$X_{k+1} = f(X_k, k) + G_k w_k \tag{4.1}$$

$$w_k \sim (0, Q_k) \tag{4.2}$$

where X_{k+1} is state vector, w_k is independent zero-mean white Gaussian noise process with covariance matrix Q_k, $f(w_k, k)$ is a state transition matrix — which is a non-linear function of states, and G_k is a system noise coupling matrix.

The measurement model can be represented as:

$$\boldsymbol{Z_{k+1} = h_{k+1}(X_{k+1}, k+1) + v_{k+1}} \tag{4.3}$$

$$v_k \sim (0, R_k) \tag{4.4}$$

where Z_{k+1} is the measurement vector, v_k is the independent zero-mean white Gaussian noise process with covariance matrix R_k, and $h_{k+1}(X_{k+1}, k+1)$ is a non-linear measurement matrix.

KF is initialized as follows:

$$\hat{X}_0^+ = E(X_0) \tag{4.5}$$

$$P_0^+ = E\left[\left(X_0 - \hat{X}_0^+\right)\left(X_0 - \hat{X}_0^+\right)^T\right] \tag{4.6}$$

where P_0^+ is the initial *a posteriori* error covariance matrix of the predicted state $\widehat{X}$.

State vector and its error covariance matrix are projected in time as:

$$\hat{X}_{k+1}^{-} = f(\hat{X}_{k}^{-}, k) \tag{4.7}$$

$$P_{k+1}^{-} = \Phi_k P_k^{+} \Phi_k^{T} + G_k Q_k G_k^{T} \tag{4.8}$$

in this instance:

$$\Phi_k = \left.\frac{\partial f(X,k)}{\partial X}\right|_{\hat{X}_k^{+}} \tag{4.9}$$

KF gain *K* is computed and used to weight external measurements to update the state vector *X* and its error covariance *P* as follows:

$$K_{k+1} = P_{k+1}^{-} H_{k+1}^{T} (H_{k+1} P_{k+1}^{-} H_{k+1}^{T} + R_{k+1})^{-1} \tag{4.10}$$

$$\hat{X}_{k+1}^{+} = \hat{X}_{k+1}^{-} + K_{k+1}\left[Z_{k+1} - h\left(\hat{X}_{k+1}^{-}, k+1\right)\right] \tag{4.11}$$

$$P_{k+1}^{+} = (I - K_{k+1} H_{k+1}) P_{k+1}^{-} \tag{4.12}$$

where:

$$H_k = \left.\frac{\partial h(X,k)}{\partial X}\right|_{X_k^{-}} \tag{4.13}$$

In this research the developed integrated tightly coupled PPP/RISS system was built through EKF.

4.4 **Developed GPS PPP/RISS Integration System**

Integrating PPP and RISS formed the developed system in this research. This system is built to accommodate single-frequency and dual-frequency code pseudorange as well as dual-frequency code and carrier GPS measurements. In this research, single-frequency code PPP/RISS (SF-code-

PPP/RISS), dual-frequency code PPP/RISS (DF-code-PPP/RISS), smoothed dual-frequency code PPP/RISS (SDF-Code-PPP/RISS), and dual-frequency code and carrier-phase PPP/RISS (code-carrier-PPP/RISS) were considered. For all of these combinations, except for ionospheric delays, all other GPS errors and biases were rigorously corrected and treated as in GPS PPP before being used as an update source for the RISS. For single-frequency code measurements, the ionosphere was corrected using the NOAA ion grid files, while ionosphere-free linear combinations were applied to account for the ionosphere delay in other combinations. In the following section, a description of the model is provided. The integration architecture is tightly coupled and the estimation method is EKF. Figure 5.3 shows a flow chart of the developed tightly coupled PPP/RISS system.

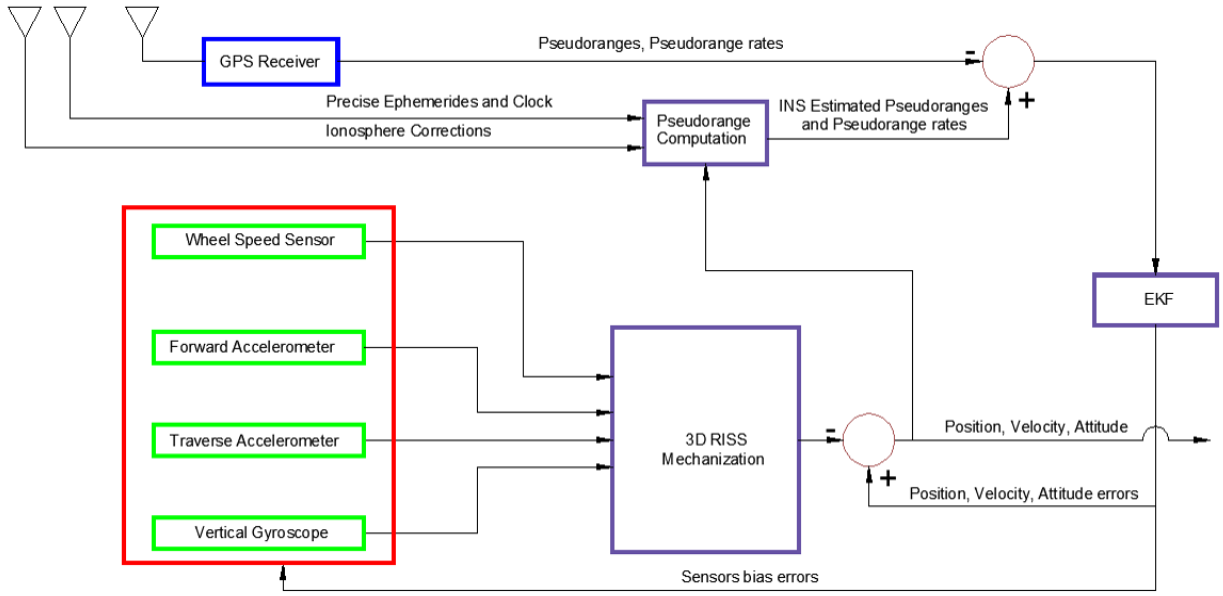

Figure 4.4 PPP/RISS Tightly Coupled Navigation System

4.4.1 Tightly-Coupled PPP/RISS Measurement Model

The measurement is the relation between the measurements and system state. It is given at time *k* as follows:

$$\delta Z_k = H_k \delta X_k + \varepsilon_k \tag{4.14}$$

where δZ_k is measurement, δX_k is the system state, H_k is the design matrix, and ε_k is system noise.

4.4.2 Tightly-Coupled PPP/RISS System Model

The state vector of the developed PPP/RISS model is:

$$\begin{aligned} \delta X_R = [&\delta\varphi,\ \delta\lambda,\ \delta h,\ \delta v_e,\ \delta v_n,\ \delta v_u,\ \delta A,\ \delta r,\ \delta p,\ \delta v_o,\ \delta a_o, \\ &\delta b_z, \delta b_X, \delta b_Y, \delta dT_b,\ \delta dT_d\ ] \end{aligned} \tag{4.15}$$

where $\delta\varphi,\ \delta\lambda,\ \delta h,\ \delta v_e,\ \delta v_n,\ \delta v_u,\ \delta A,\ \delta r,\ \delta p,\ \delta a_o,\ \delta b_z\ , \delta b_X, \delta b_Y, \delta dT_b,$ and δdT_d are the errors in: latitude; longitude; altitude; east velocity; north velocity; upward velocity; azimuth; pitch; roll; acceleration derived from wheel rotation sensor measurements and gyroscope bias, respectively; X-axis accelerometer; Y-axis accelerometer; GPS receiver clock bias; and GPS receiver clock drift.

To get the rate of change of the error states for the RISS system model, motion equations were linearized through Taylor's series expansion and only the first order terms were retained. For detail derivations of the linearization step, refer to Atia et al. (2014).

4.4.2.1 System Dynamic Matrix

The dynamic matrix is formed from the linearization of the motion's equations (Karamat, 2014). The elements of the system dynamic matrix are the coefficients of the elements of state vector. The following is a step-by-step formation of the system dynamic matrix.

4.4.2.2 Latitude

The time rate change of the latitude is given as:

$$\dot{\varphi} = \frac{v_n}{R_M + h} \tag{4.16}$$

After applying Taylor's series the above and arranging the terms, we get:

$$\delta\dot{\varphi} = \left(\frac{1}{R_M + h}\right)\delta v_n \tag{4.17}$$

4.4.2.3 Longitude

The time rate of change of the longitude is given as:

$$\dot{\lambda} = \frac{v_e}{(R_N + h)\cos\varphi} \tag{4.18}$$

Again, applying Taylor's series for the rate of change of the longitude equation, and arranging the terms, we get:

$$\delta\dot{\lambda} = \left(\frac{v_e}{(R_N + h)\cos\varphi}\right)\delta v_e + \left(\frac{v_e \tan\varphi}{(R_N + h)\cos\varphi}\right)\delta\varphi \tag{4.19}$$

4.4.2.4 Altitude

The rate of change of the altitude is directly related to the vertical velocity as:

$$\dot{h} = v_u \tag{4.20}$$

Linearization of the rate of change of the altitude results in:

$$\dot{h} = \delta v_u \tag{4.21}$$

4.4.2.5 East velocity

East velocity is obtained from the forward velocity of the vehicle speed odometer. It is expressed as:

$$v_e = v_o \sin A \cos p \tag{4.22}$$

The linearization result for the above equation is written as:

$$\begin{aligned} \delta \dot{v}_e = {} & \sin A \cos p \, \delta a_O + a_O \cos A \cos p \, \delta A \\ & - \left(\omega_Z - b_Z - \omega^e \sin \varphi - \frac{v_e \tan \varphi}{R_N + h} \right) \delta v_n + v_n \delta b_z \\ & + v_n \left(\omega^e \cos \varphi + \frac{v_e \sec^2 \varphi}{R_N + h} \right) \delta \varphi + \left(\frac{v_n \tan \varphi}{R_N + h} \right) \delta v_e \end{aligned} \tag{4.23}$$

4.4.2.6 North velocity

Similarly to east velocity, north velocity is obtained from the forward velocity of the vehicle speed odometer, as in:

$$v_n = v_O \cos A \cos p \tag{4.24}$$

And the final arrangement of the linearization is presented as:

$$\delta \dot{v}_n = \cos A \cos p \, \delta a_O - a_O \sin A \cos p \, \delta A + \left(\omega_Z - b_Z - \omega^e \sin \varphi - \frac{v_e \tan \varphi}{R_N + h}\right) \delta v_e - v_e \delta b_Z - v_e \left(\omega^e \cos \varphi + \frac{v_e \sec^2 \varphi}{R_N + h}\right) \delta \varphi + \tag{4.25}$$

4.4.2.7 Up velocity

Up velocity is obtained as the vertical resolution of the forward velocity, and in this resolution the pitch angle is used as follows:

$$v_u = v_O \sin p \tag{4.26}$$

After differentiating the above equation with respect to time and applying Taylor's series for linearization, we get:

$$\delta \dot{v}_u = (\sin p) \, \delta a_O \tag{4.27}$$

4.4.2.8 Attitude

Azimuth is the only attitude component that is included in the state vector. Azimuth's time rate of change is expressed as:

$$\dot{A} = -\left[(\omega_Z - b_Z) - \omega^e \sin \varphi - \left(\frac{v_e \tan \varphi}{R_N + h}\right)\right] \tag{4.28}$$

The linearization of the azimuth time rate of change leads to the following relationship:

$$\delta \dot{A} = \delta b_Z + \left(\omega^e \cos \varphi + \frac{v_e \sec^2 \varphi}{R_N + h}\right) \delta \varphi + \left(\frac{\tan \varphi}{R_N + h}\right) \delta v_e \tag{4.29}$$

4.4.2.9 Gyroscope stochastic errors

The gyroscope stochastic error is modelled by the first-order Gauss-Markov process and is presented as follows:

$$\delta \dot{b}_Z = -\beta_Z \delta b_Z + \sqrt{2\beta_Z \sigma_Z^2}\omega(t) \tag{4.30}$$

4.4.2.10 Wheel rotation sensor stochastic model

Similarly to the gyroscope wheel rotation sensor, random errors are also modelled as first-order Gauss-Markov processes, which are expressed as:

$$\delta \dot{a}_o = -\beta_o \delta a_o + \sqrt{2\beta_o \sigma_o^2}\omega(t) \tag{4.31}$$

In the above two equations, β and σ are the reciprocal of the correlation time and the standard deviation, respectively.

The system dynamic matrix for the developed PPP/RISS integrated system is created from the coefficients of equations (4.17), (4.19), (4.21), (4.23), (4.25), (4.27), (4.29), (4.30), and (4.31), so the no-zero element of the F matrix are populated as:

$$F(1,5) = \left(\frac{1}{R_M + h}\right)$$

$$F(2,1) = \left(\frac{v_e \tan\varphi}{(R_N + h)\cos\varphi}\right)$$

$$F(2,4) = \left(\frac{1}{(R_N + h)\cos\varphi}\right)$$

$$F(3,6) = 1$$

$$F(4,1) = v_n\left(\omega^e \cos\varphi + \frac{v_e \sec^2\varphi}{R_N + h}\right)$$

$$F(4,4) = \left(\frac{v_n \tan\varphi}{R_N + h}\right)$$

$$F(4,5) = -\left(\omega_Z - b_Z - \omega^e \sin\varphi - \frac{v_e \tan\varphi}{R_N + h}\right)$$

$$F(4,7) = a_O \cos A \cos p$$

$$F(4,8) = \sin A \cos p$$

$$F(4,9) = v_n$$

$$F(5,1) = -v_e\left(\omega^e \cos\varphi + \frac{v_e \sec^2\varphi}{R_N + h}\right)$$

$$F(5,4) = \left(\omega_Z - b_Z - \omega^e \sin\varphi - \frac{v_e \tan\varphi}{R_N + h}\right)$$

$$F(5,7) = -a_O \sin A \cos p$$

$$F(5,8) = \cos A \cos p$$

$$F(5,9) = -v_e$$

$$F(6,8) = (\sin p)$$

$$F(7,9) = 1$$

$$F(7,1) = \left(\omega^e \cos\varphi + \frac{v_e \sec^2\varphi}{R_N + h}\right)$$

$$F(7,4) = \left(\frac{\tan\varphi}{R_N + h}\right)$$

$$F(8,8) = -\beta_{a_od}$$

$$F(9,9) = -\beta_{w_z}$$

$$F(10,11) = 1$$

$$F(11,11) = 0$$

$$F(12,12) = -\beta_r$$

$$F(13,13) = -\beta_p$$

$$F(14,14) = -\beta_{f_x}$$

$$F(15,15) = -\beta_{f_y}$$

$$F(16,8) = 1$$

$$F(16,16) = 0$$

4.4.2.11 System Noise

System noise is represented by the G (appears in equation 4.1) matrix shown below:

$$G = \begin{bmatrix}
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & \sqrt{2\beta_o\sigma_o^2} & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \sqrt{2\beta_{\omega_z}\sigma_{\omega_z}^2} & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \sqrt{2\beta_{T_b}\sigma_{T_b}^2} & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \sqrt{2\beta_{T_d}\sigma_{T_d}^2} & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \sqrt{2\beta_o\sigma_o^2} & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \sqrt{2\beta_p\sigma_p^2} & 0 & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \sqrt{2\beta_r\sigma_r^2} & 0 & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \sqrt{2\beta_{f_x}\sigma_{f_x}^2} & 0 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & \sqrt{2\beta_{f_y}\sigma_{f_y}^2} & 0
\end{bmatrix}$$

where β is the reciprocal of the correlation time and σ is the standard deviation. The subscripts o, ω_z, T_b, T_d, p,r, f_x, and f_z stand for: odometer acceleration; Z-gyroscope measurement; receiver clock bias; receiver clock drift; pitch angle; roll angle; X-accelerometer measurement; and Y-accelerometer measurement.

4.4.3 Measurement Model

The measurement model relates the measurement to the states which are derived by taking the design matrix. In the tightly coupled GPS/INS integration, the measurements for KF are the difference in RISS-computed and GPS-measured pseudorange and pseudorange rates. These

measurements are involved in the following linearized discrete measurement models in the time domain:

$$\delta z = H\delta x + \varepsilon \tag{4.44}$$

where δz is the measurement vector, H is the measurement design matrix, δx is the state vector (for this research, the size of state vector is 16×1), and ε is the vector of random noise measurement with zero mean and covariance R. δz is a vector that is formed as the difference between RISS-computed and GPS-measured pseudoranges $\delta\rho$ and pseudorange rate $\delta\dot{\rho}$, as follows:

$$\begin{bmatrix}\delta Z_{\rho} \\ \delta Z_{\dot{\rho}}\end{bmatrix} = \begin{bmatrix}\rho_{RISS} - \rho_{GPS} \\ \dot{\rho}_{RISS} - \dot{\rho}_{GPS}\end{bmatrix} \tag{4.45}$$

where ρ_{RISS} is RISS-computed pseudorange, ρ_{GPS} is the GPS pseudorange, $\dot{\rho}_{RISS}$ is the RISS-computed pseudorange rate, and $\dot{\rho}_{GPS}$ is the GPS Doppler measurement.

A full measurement vector tightly coupled PPP-RISS integration that involves M GPS satellites is written as:

$$\begin{bmatrix}\delta z_{\rho}^{1} \\ \delta z_{\rho}^{2} \\ \vdots \\ \delta z_{\rho}^{M} \\ \delta z_{\dot{\rho}}^{1} \\ \delta z_{\dot{\rho}}^{2} \\ \vdots \\ \delta z_{\dot{\rho}}^{M}\end{bmatrix} = \begin{bmatrix}\rho_{RISS}^{1} - \rho_{GPS}^{1} \\ \rho_{RISS}^{2} - \rho_{GPS}^{2} \\ \vdots \\ \rho_{RISS}^{M} - \rho_{GPS}^{M} \\ \dot{\rho}_{RISS}^{1} - \dot{\rho}_{GPS}^{1} \\ \dot{\rho}_{RISS}^{2} - \dot{\rho}_{GPS}^{2} \\ \vdots \\ \dot{\rho}_{RISS}^{M} - \dot{\rho}_{GPS}^{M}\end{bmatrix} \tag{4.46}$$

where:

$$\rho_{GPS}^{m} = \|X - X^{m}\| - \delta b_s + T + I + dm + \delta b_r + \varepsilon_{\rho}^{m} \tag{4.47}$$

where X and X^m are the receiver and satellite position vectors, respectively, δb_r and δb_r are the receiver and satellite clock biases, respectively, T and I are the tropospheric and ionospheric delays, respectively, dm is the multipath error, and ε_{ρ}^{m} is the total effect of residual errors produced due to atmospheric delay, receiver noise, etc.

In this research we instigated the use of single- and dual-frequency GPS measurements for PPP/RISS integration. For single-frequency only L1 code measurements are used, while for dual-frequency measurements both code as well as code carrier measurements.

4.4.3.1 GPS Dual-Frequency Measurements

The dual-frequency GPS code (P_1 and P_2) and carrier-phase ($\Phi_1$and Φ_2) measurements were used to generate ionosphere-free code and code and carrier pseudoranges, as follows:

$$P(k) = \frac{(f_1^2 P_1(t)\lambda_1 - f_2^2 P_2(t)\lambda_2)}{f_1^2 - f_2^2} \tag{4.48}$$

$$P(k-1) = \frac{(f_1^2 P_1(t-1)\lambda_1 - f_2^2 P_2(t-1)\lambda_2)}{f_1^2 - f_2^2} \tag{4.49}$$

$$P_{diff}(k) = P(k) - P(k-1) \tag{4.50}$$

$$\Phi(k) = \frac{(f_1^2 \Phi_1(k)\lambda_1 - f_2^2 \Phi_2(k)\lambda_2)}{f_1^2 - f_2^2} \tag{4.51}$$

$$\Phi(k-1) = \frac{(f_1^2 \Phi_1(k-1)\lambda_1 - f_2^2 \Phi_2(k-1)\lambda_2)}{f_1^2 - f_2^2} \tag{4.52}$$

$$\Phi_{diff}(k) = \Phi(k) - \Phi(k-1) \tag{4.53}$$

$$\Delta = P_{diff} - \Phi_{diff} \tag{4.54}$$

In this research, smoothed and unsmoothed ionosphere codes and code and carriers were used. The unsmoothed ionosphere-free code only pseudorange, $P(k)$ was computed in equation (4.48). The smoothed ionosphere-free code pseudorange, P_{flt} , is expressed as:

$$P_{flt}(k) = N(1) + COR_{flt} + N_{COR}(k) + \sum_{i=1}^{i=k} ION(i) \tag{4.55}$$

The filtered ionosphere-free code and carrier-phase range, Φ_{flt} , is expressed as:

$$\Phi_{flt}(k) = N(1) + \sum_{i=1}^{i=k} ION(i) \tag{4.56}$$

$$N_{COR}(k) = \frac{\left(\sum_{i=1}^{i=k} N(i) - COR_{flt}\right)}{k} \tag{4.57}$$

$$\sum_{i=1}^{i=k} N(i) = \sum_{i=1}^{i=k} \Delta(i) \tag{4.58}$$

$$\sum_{i=1}^{i=k} ION\,(i) = \sum_{i=1}^{i=k} \Phi_{diff}\,(i) \tag{4.59}$$

where $N_{COR}(k)$ is the last correction to the ambiguity, $\sum_{i=1}^{i=k} N(i)$ is sum of ambiguities, and $\sum_{i=1}^{i=k} ION\ (i)$ is sum of ionospheric carrier/code difference from the initial epoch.

4.4.3.2 GPS Single-Frequency Measurement

In addition to the different combinations of the dual-frequency GPS measurement presented above, we also used single-frequency code data. The *L1* C/A code was to obtain a correct GPS measurement (ρ_{GPS}^{m}) for the measurements model vector, as follows:

$$\rho_{GPS}^{m} = C_1 - \delta b_s + T + I + Sag + Rel + HWD + dm + \delta b_r + \varepsilon_{\rho}^{m} \tag{4.60}$$

RISS-computed pseudorange is expressed as:

$$\rho_{RISS}^{m} = \|X_{RISS} - X^{m}\| \tag{4.61}$$

where X_{RISS} is the position in ECEF Cartesian coordinates of the IMU calculated from the RISS mechanization's position output.

By applying Taylor's series to the difference between the RISS estimate and GPS pseudorange, a linear relation was established through which to relate the measurements to errors, as follows:

$$\delta Z_{\rho}^{m} = I_{RISS}^{m} \cdot (X_{RISS} - X^{m}) - \delta b_r + \delta b_d + \tilde{\epsilon}_{\rho}^{m} \tag{4.62}$$

where I_{RISS}^{m} is the line-of-sight unit vector from m^{th} satellite to the receiver antenna base on the output of RISS mechanization. This unit vector is expressed as:

$$I_{RISS}^{m} = \frac{X_{RISS} - X^{m}}{\|X_{RISS} - X^{m}\|} \tag{4.63}$$

by using a matrix form and applying equation (5.62) for the M satellite, we get:

$$\begin{bmatrix} \delta Z_\rho^1 \\ \delta Z_\rho^1 \\ \vdots \\ \delta Z_\rho^M \end{bmatrix} = \begin{bmatrix} I_{x,RISS}^1 & I_{y,RISS}^1 & I_{z,RISS}^1 \\ I_{x,RISS}^2 & I_{y,RISS}^2 & I_{z,RISS}^2 \\ \vdots & \vdots & \vdots \\ I_{x,RISS}^M & I_{y,RISS}^M & I_{z,RISS}^M \end{bmatrix} \begin{bmatrix} \delta x \\ \delta y \\ \delta z \end{bmatrix} - \begin{bmatrix} \delta b_r \\ \delta b_r \\ \vdots \\ \delta b_r \end{bmatrix} + \begin{bmatrix} \tilde{\epsilon}_\rho^1 \\ \tilde{\epsilon}_\rho^2 \\ \vdots \\ \tilde{\epsilon}_\rho^M \end{bmatrix} \tag{4.64}$$

where $\delta x = X_{RISS} - X,\ \delta y = Y_{RISS} - Y,$ and $\delta z = Z_{RISS} - Z.$

Positions are in geodetic coordinates, so, to transform them into ECEF Cartesian coordinates, the following relations were used:

$$\begin{bmatrix} x \\ y \\ z \end{bmatrix} = \begin{bmatrix} (R_N + h)\cos\varphi\cos\lambda \\ (R_N + h)\cos\varphi\sin\lambda \\ (R_N(1 - e^2) + h)\sin\varphi \end{bmatrix} \tag{4.65}$$

by applying Taylor's series for the above transformation relations and ignoring higher terms, we got:

$$\begin{bmatrix} \delta x \\ \delta y \\ \delta z \end{bmatrix} = \begin{bmatrix} -(R_N + h)\sin\varphi\cos\lambda & -(R_N + h)\cos\varphi\sin\lambda & \cos\varphi\cos\lambda \\ -(R_N + h)\sin\varphi\sin\lambda & (R_N + h)\cos\varphi\cos\lambda & \cos\varphi\sin\lambda \\ (R_N(1 - e^2) + h)\cos\varphi & 0 & \sin\varphi \end{bmatrix} \begin{bmatrix} \delta\varphi \\ \delta\lambda \\ \delta h \end{bmatrix} \tag{4.66}$$

GPS pseudorange rates or Doppler measurements are related to the relative velocity between the satellite and the receiver projected onto the direction of the unit vector from the satellite to the receiver, as in the following equation:

$$\dot{\rho}_{GPS}^m = (V - V^m) \cdot I^m + \delta d_r + \varepsilon_\rho^m \tag{4.67}$$

where V^m is the m^{th} satellite velocity in the ECEF frame, V is the true receiver velocity also in the ECF farme, δd_r is the receiver clock drift, ε_ρ^m is the error in observation, and I^m is the true line-of-sight unit vector pointing from the m^{th} satellite to the receiver, shown in:

$$I^m = \frac{X - X^m}{\|X - X^m\|} \tag{4.68}$$

This equation can be rewritten in more detail:

$$\dot{\rho}^m_{GPS} = (V_x - V_x^m) \cdot I_x^m + \left(V_y - V_y^m\right) \cdot I_y^m + (V_z - V_z^m) \cdot I_z^m + \delta d_r + \varepsilon_\rho^m \tag{4.69}$$

RISS-estimated pseudorange rate can be expressed as:

$$\dot{\rho}^m_{RISS} = (V_{RISS} - V^m) \cdot I^m_{RISS} \tag{4.70}$$

where V_{RISS} is the RISS-estimated vehicle velocity referenced to ECEF Cartesian coordinates. Similarly to the GPS pseudorange rate equation (5.69), the RISS pseudorange rate equation can also be written as:

$$\begin{aligned}\dot{\rho}^m_{RISS} = \left(V_{x,RISS} - V_x^m\right) \cdot I_x^m + \left(V_{y,RISS} - V_y^m\right) \cdot I_y^m + \left(V_{z,RISS} - V_z^m\right) \cdot I_z^m \\ + \delta d_r + \varepsilon_\rho^m\end{aligned} \tag{4.71}$$

where $V_{x,RISS}$, $V_{y,RISS}$, and $V_{z,RISS}$ are the RISS-estimated velocities for the receiver referenced to ECEF system.

The next step was to compute the pseudorange rate differences between RISS and GPS to further populate the system measurements vector:

$$\begin{aligned}\dot{\rho}^m_{RISS} - \rho^m_{GPS} = \left(V_{x,RISS} - V_x^m\right) \cdot I_x^m + \left(V_{y,RISS} - V_y\right) \cdot I_y^m + \left(V_{z,RISS} - V_y\right) \\ \cdot I_z^m + \delta d_r + \varepsilon_\rho^m\end{aligned} \tag{4.72}$$

This equation (5.72) can be rewritten as:

$$\delta z_{\dot{\rho}}^{m} = \delta V_x \cdot I_{x,RISS}^{m} + \delta V_y \cdot I_{y,RISS}^{m} + \delta V_z \cdot I_{z,RISS}^{m} + \delta d_r + \varepsilon_{\rho}^{m} \tag{4.73}$$

Now, writing (5.73) in matrix form:

$$\delta z_{\dot{\rho}}^{m} = \begin{bmatrix} I_{x,RISS}^{m} & I_{y,RISS}^{m} & I_{z,RISS}^{m} \end{bmatrix} \begin{bmatrix} \delta V_x \\ \delta V_y \\ \delta V_z \end{bmatrix} + \delta d_r + \varepsilon_{\rho}^{m} \tag{4.74}$$

Staking the above equation for M GPS satellites, we got:

$$\begin{bmatrix} \delta z_{\dot{\rho}}^{1} \\ \delta z_{\dot{\rho}}^{2} \\ \vdots \\ \delta z_{\dot{\rho}}^{M} \end{bmatrix} = \begin{bmatrix} I_{x,RISS}^{1} & I_{y,RISS}^{1} & I_{z,RISS}^{1} \\ I_{x,RISS}^{2} & I_{y,RISS}^{2} & I_{z,RISS}^{2} \\ \vdots & \vdots & \vdots \\ I_{x,RISS}^{M} & I_{y,RISS}^{M} & I_{z,RISS}^{M} \end{bmatrix} \begin{bmatrix} \delta V_x \\ \delta V_y \\ \delta V_z \end{bmatrix} + \begin{bmatrix} \delta d_r \\ \delta d_r \\ \vdots \\ \delta d_r \end{bmatrix} + \begin{bmatrix} \varepsilon_{\rho}^{1} \\ \varepsilon_{\rho}^{2} \\ \vdots \\ \varepsilon_{\rho}^{M} \end{bmatrix} \tag{4.75}$$

Now, we needed to include the velocity vector in the LLF in the above system. To do that, we needed to multiply the velocity vector in ECEF frame by the rotation matrix that transforms between the ECEF and LLF systems. The rotation matrix is as follows:

$$R_l^e = \begin{bmatrix} -\sin\lambda & -\sin\varphi\cos\lambda & \cos\varphi\cos\lambda \\ \cos\lambda & -\sin\varphi\sin\lambda & \cos\varphi\sin\lambda \\ 0 & \cos\varphi & \sin\varphi \end{bmatrix} \tag{4.76}$$

where φ and λ are the latitude and longitude of the receiver.

Next, the velocity vector in LLF is as follows:

$$\begin{bmatrix}\delta V_x\\ \delta V_y\\ \delta V_z\end{bmatrix}=\begin{bmatrix}-\sin\lambda & -\sin\varphi\cos\lambda & \cos\varphi\cos\lambda\\ \cos\lambda & -\sin\varphi\sin\lambda & \cos\varphi\sin\lambda\\ 0 & \cos\varphi & \sin\varphi\end{bmatrix}\begin{bmatrix}\delta V_e\\ \delta V_n\\ \delta V_u\end{bmatrix} \tag{4.77}$$

So the pseudorange rate measurements with velocities in LLF are:

$$\begin{bmatrix}\delta z_{\dot\rho}^1\\ \delta z_{\dot\rho}^2\\ \vdots\\ \delta z_{\dot\rho}^M\end{bmatrix}$$

$$=\begin{bmatrix}I_{x,RISS}^1 & I_{y,RISS}^1 & I_{z,RISS}^1\\ I_{x,RISS}^2 & I_{y,RISS}^2 & I_{z,RISS}^2\\ \vdots & \vdots & \vdots\\ I_{x,RISS}^M & I_{y,RISS}^M & I_{z,RISS}^M\end{bmatrix}\begin{bmatrix}-\sin\lambda & -\sin\varphi\cos\lambda & \cos\varphi\cos\lambda\\ \cos\lambda & -\sin\varphi\sin\lambda & \cos\varphi\sin\lambda\\ 0 & \cos\varphi & \sin\varphi\end{bmatrix}\begin{bmatrix}\delta V_e\\ \delta V_n\\ \delta V_u\end{bmatrix} \tag{4.78}$$

$$+\begin{bmatrix}\delta d_r\\ \delta d_r\\ \vdots\\ \delta d_r\end{bmatrix}+\begin{bmatrix}\varepsilon_\rho^1\\ \varepsilon_\rho^2\\ \vdots\\ \varepsilon_\rho^M\end{bmatrix}$$

The above system can be rewritten in matrix notations as:

$$\delta z_{\dot\rho}^m = G_{M\times3}R_l^e\delta V_{3\times1}^l+\delta d_{r,M\times1}+\varepsilon_{\rho,M\times1}^m \tag{4.79}$$

and by introducing:

$$H_{M\times3}^{\dot\rho}=G_{M\times3}R_l^e\delta V_{3\times1}^l \tag{4.80}$$

$$\delta z_{\dot\rho}^m = H_{M\times3}^{\dot\rho}\delta V_{3\times1}^l+\delta d_{r,M\times1}+\varepsilon_{\rho,M\times1}^m \tag{4.81}$$

Now, by combining the pseudorange and pseudorange rate differences and overall measurements, the model can be presented as:

$$\begin{bmatrix} \delta z_{\rho}^{m} \\ \delta z_{\dot{\rho}}^{m} \end{bmatrix} = \begin{bmatrix} H_{M\times 3}^{\rho} & 0_{M\times 3} & 0_{M\times 3} & 1_{M\times 3} & 0_{M\times 3} \\ 0_{M\times 3} & H_{M\times 3}^{\dot{\rho}} & 0_{M\times 3} & 0_{M\times 3} & 1_{M\times 3} \end{bmatrix} + \begin{bmatrix} \varepsilon_{\rho,M\times 1}^{m} \\ \varepsilon_{\dot{\rho},M\times 1}^{m} \end{bmatrix} \qquad (4.82)$$

A summary of the developed EKF PPP/RISS model characteristics is as follows:

1) It is a closed-loop system in which the navigation state is measured by the estimated position, velocity, and attitude errors.
2) The gyroscope bias is contagiously corrected by the estimated bias error, which helps in maintaining good accuracy during GPS outage.
3) The system is based on EKF, which estimates errors to correct navigation. This ensures accurate navigation states — and, consequently, linearization is executed around the correct state.
4) The system integration scheme is tightly coupled, which allow the system to benefit from any available satellites.

Chapter 5

5 Results and Analysis

The performance of the developed tightly coupled PPP/RISS algorithms was tested on real data collected through various road trajectories conducted under different navigational conditions in a van-type land vehicle. In this chapter, results of processing data from three trajectories are presented. Trajectories are name as: Trajectory 1, Trajectory 2, and Trajectory 3. The state vector of the developed integrated system includes position, velocity, attitude, sensor biases, and receiver's clock bias and drift; however, the results presentation is focused on the positioning results. Figure 5.1 shows a table installed in place of the rear seat of the van that used to travel the trajectories. Attached to the table is part of the equipment used to collect the data.

Figure 5.1 Part of the data collection equipment placed inside the van

In the following subsections, more details are given to help the reader understand the experimental setup details.

5.1 RISS Configuration

To target commercial land vehicle applications with low-cost sensors, the developed system was tested using an inexpensive MEMS-based IMU unit (Crossbow brand) that carries the number IMU300CC (Crossbow, 2007) along with odometry data that includes van speed measurements. The Crossbow IMU is a "six-degree-of-freedom" inertial system that uses solid-state devices to measure angular rates and linear accelerations. The Crossbow IMU300CC is capable of measuring X-, Y- and Z-axis accelerations and rate of change of rotations around any of these three axes. However, for RISS, data from only the vertical gyroscope and forward and lateral accelerometers was used. The speed measurements were obtained from the vehicle built-in wheel sensor and collected through an OBO II interface using a data logger called "CarChip" (CarChip, 2013).

5.2 Reference Solution

To assess and evaluate the results, the developed system performance was compared to a high-end tactical grade INS, which was integrated with GPS to serve as a reference solution. In Trajectory 1 and 3, the reference system was based on the Honeywell HG1700 AG17 high-end tactical-grade IMU integrated with the NovAtel OEM4 GPS receiver (NovAtel, 2005), using an off-the-shelf assembly: the OEM4-G2 ProPak_G2plus Synchronized Position Attitude Navigation (SPAN) unit developed by NovAtel (SPAN, 2005). The NovAtel units provide a tightly coupled INS/GPS navigation solution that was used as a reference to compare the performance and the effectiveness of the developed methods. Table B.1 lists the major specification of IMU300CC and HG1700

IMUs. In Trajectory 2, the reference system was based on the NovAtel IMU-CPT high-end, tactical-grade IMU integrated with the NovAtel OEM4 GPS receiver (NovAtel, 2005) using an off-the-shelf assembly: the OEM4-G2 ProPak_G2plus Synchronized Position Attitude Navigation (SPAN) unit developed by NovAtel (NovAtel, 2012). Detailed sensors' specifications are presented in appendix B of this thesis. The reference solution processed thorough NovAtel Inertial Explorer (IE), a powerful post-processing with various configuration for GPS/INS integration schemes and GPS mode. In this work, IE is used with tightly-coupled DGPS/INS configuration.

5.3 Time Synchronization

Both systems (the low-cost tightly coupled PPP/RISS developed system and the NovAtel high-end reference system) were installed inside the van. The GPS time was used as the reference time, so data from all sensors, including IMU and speed measurements, was synchronized according to the GPS time tag. Also, in the processing stage, the GPS time tag was used to synchronize all measurements and apply the developed filtering algorithms.

5.4 Road Test Trajectories

Several real trajectories were carried out to collect data for testing the developed tightly coupled PPP/RISS integrated algorithm. Three trajectories were selected for this work to show the performance of the developed algorithm under different navigation conditions and compare it with earlier algorithms. These three trajectories are shown later in Figures 5.2, 5.20, and 5.30. The selection of these trajectories was based on the criteria that they should include driving conditions a driver might encounter when driving in urban and suburban areas and on highways. The selection

of the trajectories ensured speed variations from zero to 100 km per hr, so sudden stops, accelerations, and sharp as well as smooth turns were experienced during the trajectories.

5.5 Trajectory 1

The first trajectory was carried out in Kingston, Ontario, Canada. The trajectory route is an urban/suburban environment that included the center of the city. This road test was performed for a duration of about 30 minutes, during which a distance of 30 km was travelled while in continuous navigation. Figure 6.2 shows a plot of trajectory drawn on a google map. The main check for the developed integrated system's accuracy is during a GPS signal outage, which was intentionally introduced for a number of outages during the trajectory. Figure 6.2 also shows the locations of the outages in this trajectory, which almost covers a full level of the dynamics that could be encountered using this navigation equipment. Outages were selected to mimic conditions of a typical trip, such as during turns, straight portions, slopes, stops, and at different levels of speed.

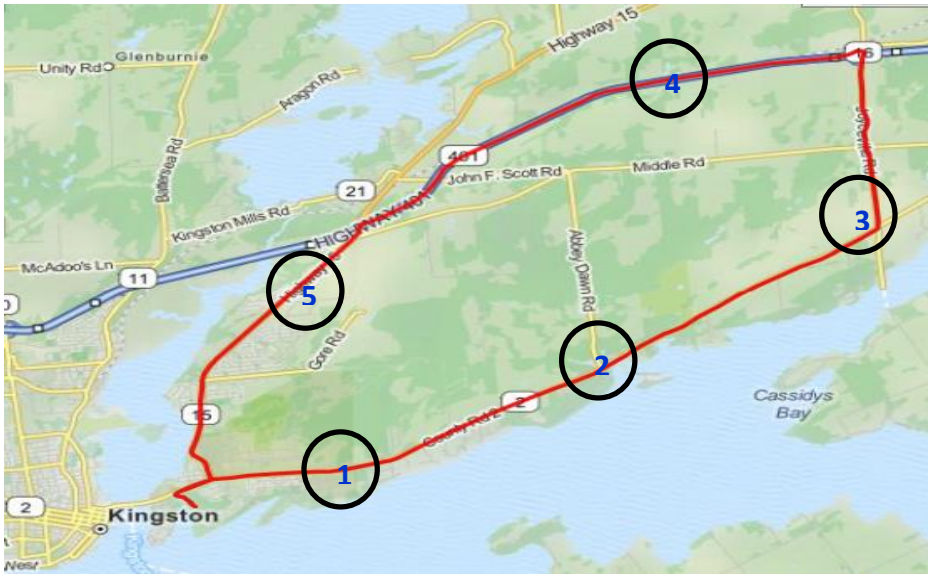

Figure 5.2 Trajectory 1

The trajectory includes suburban roads (60 to 80 km per hr speed) as well as a highway (100 km per hr speed). Figure 5.3 depicts the change in the vehicle speed during the period of the trajectory.

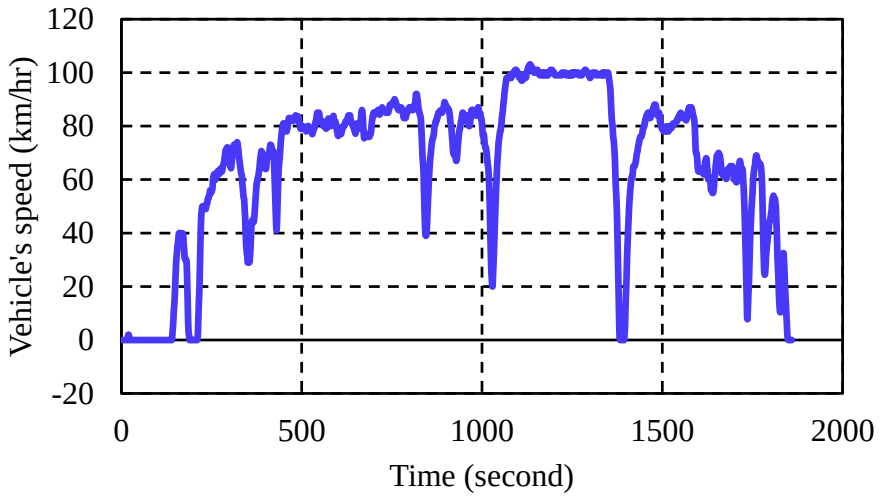

Figure 5.3 Vehicle Speed for Trajectory 1

5.5.1 Data Preprocessing

Data pre-processing is meant to examine and evaluate the quality of the GPS data. To preprocess data for trajectories in this thesis, TEQC, a quality check software developed by UNAVCO, is used to process the GPS observations and navigation data. Appendix B.1 includes a detailed quality check report for the GPS data for this trajectory. Data checking includes checking dilution of precision (DOP) for the whole trajectory, multipath, and signal-to-noise ratio for each satellite. Figure 6.4 shows satellites availability during trajectory, Figure 5.5 shows DOP values. Figures 6.6 and 6.7 show sky views for multipath MP1 and MP2. Figures 5.8 and 5.9 show sky views for the SNR on L1 and L2, respectively.

For this trajectory, GDOP values indicate good satellite geometry during the whole trajectory. Multipath plots show low multipath except for satellite G12; the value reached 1.5 m at the end of the trajectory for both MP1 and MP2, and, also, satellite G19 showed a high value of MP2 all over the trajectory. SNR values showed relatively good SNR for L1 and relatively bad SNR for L2. The variability of SNR was due to the receiver being in high dynamic during land-vehicle navigation, which affects the strength of the GPS signal.

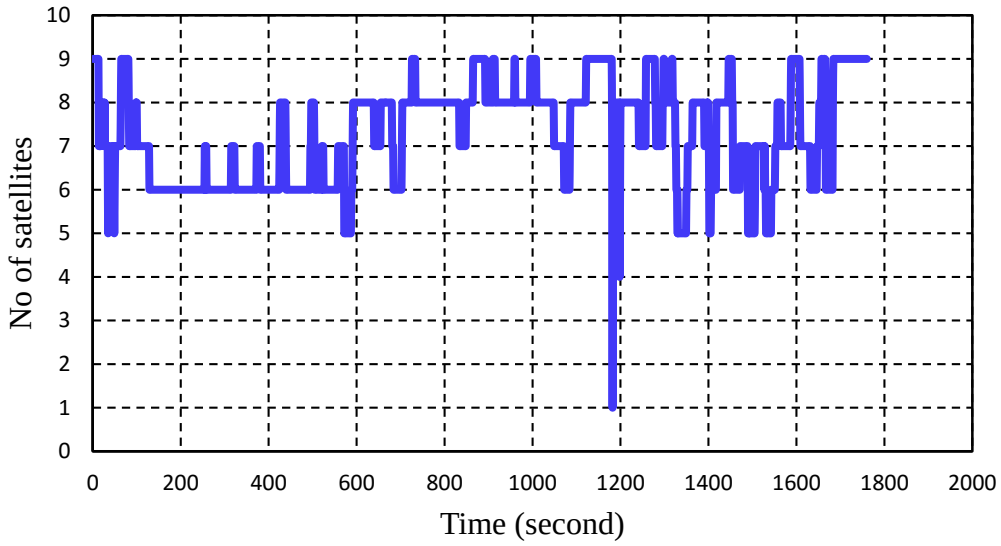

Figure 5.4 Satellites availability for trajectory 1

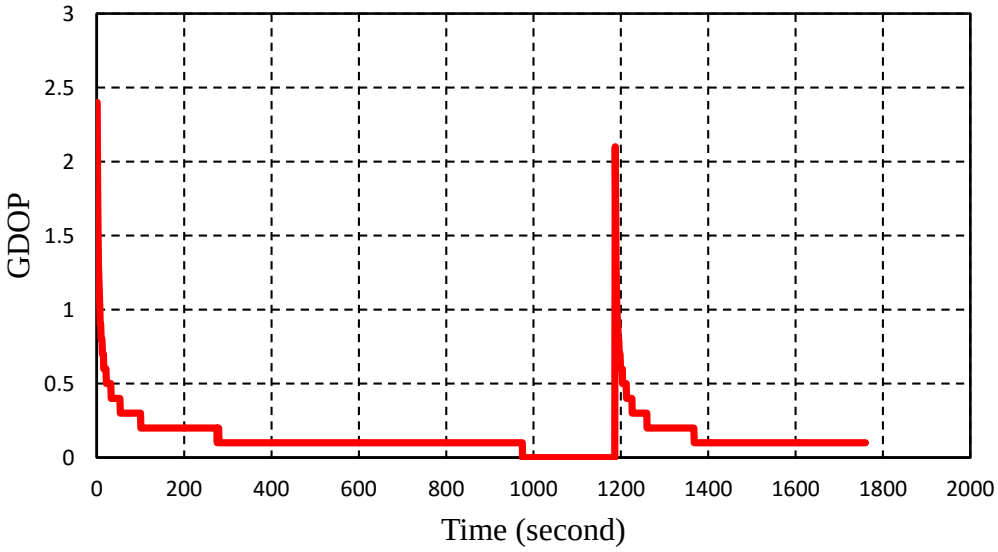

Figure 5.5 GDOP during Trajectory 1

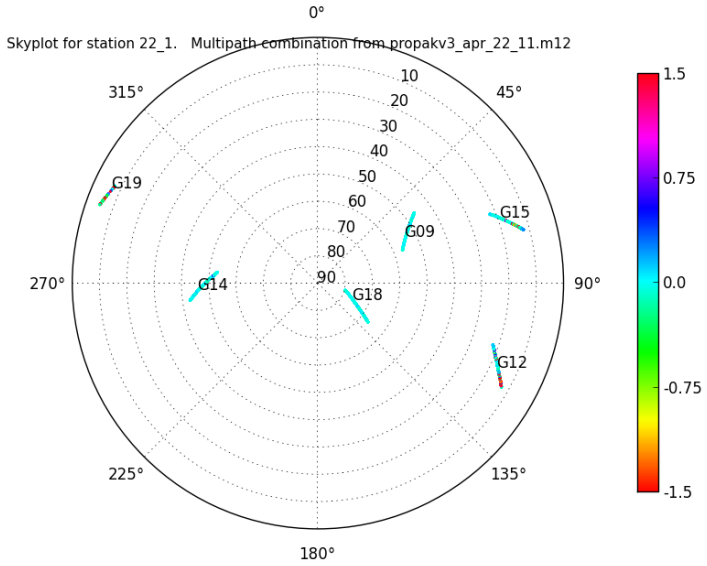

Figure 5.6 Sky View to Show Multipath MP1

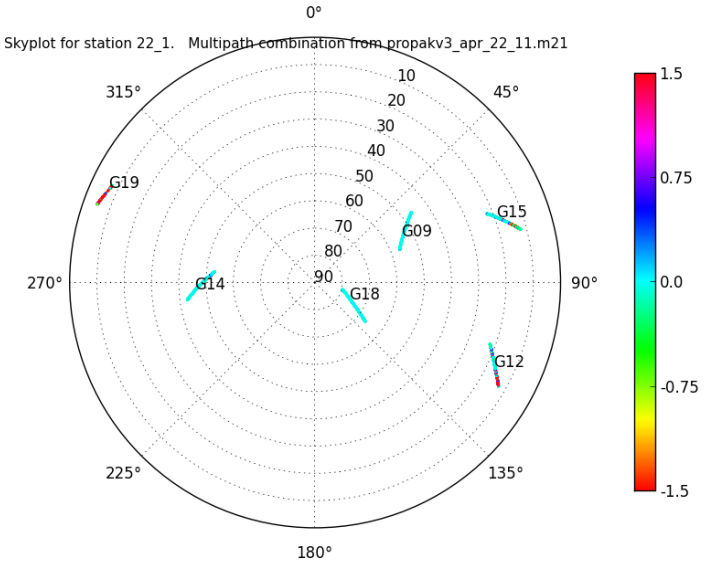

Figure 5.7 Sky View to Show Multipath MP2

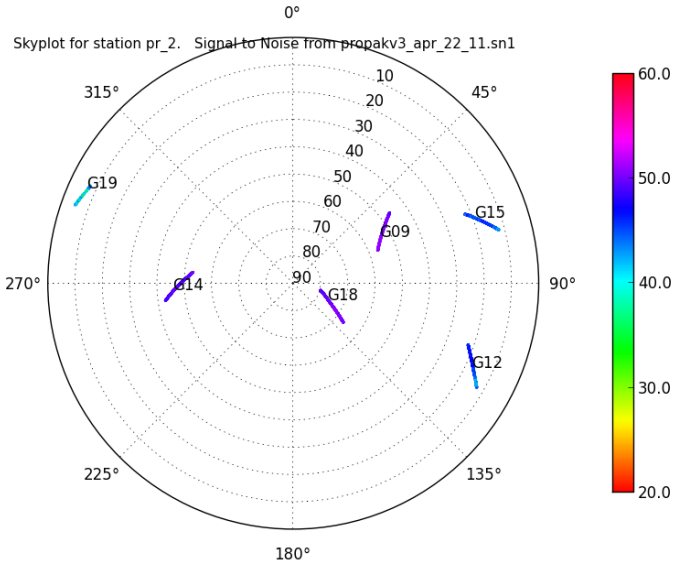

Figure 5.8 Sky View to Show L1 SNR

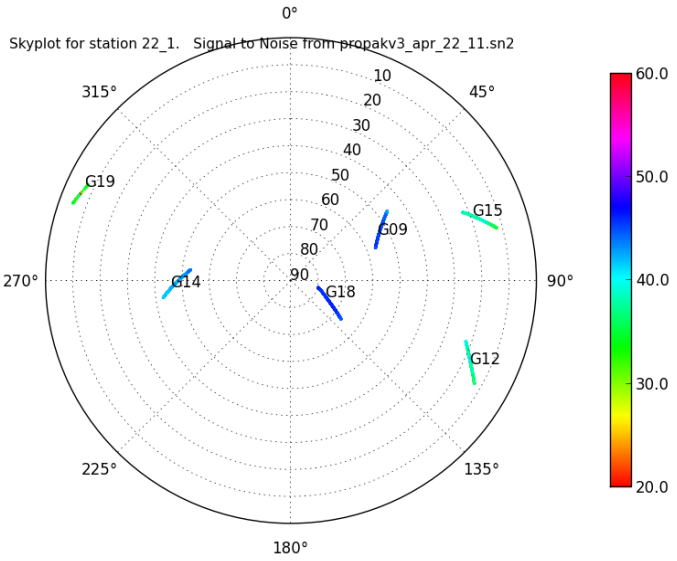

Figure 5.9 Sky View to Show L2 SNR

5.6 Performance Evaluation

The performance of the developed algorithm was tested during the whole trajectory against a reference integrated DGPS/INS solution. In addition, we also compared solutions from the developed algorithm to code-PPP and code- and carrier-PPP solutions. Evaluation of all the three algorithms positional accuracy was tested by means of the computed RMS errors for latitude, longitude, altitude, and two-dimensional and three-dimensional positions, which is shown in Figure 5.12. In all the cases, EKF-Code-Carrier-PPP/RISS had the best performance and EKF-SF-Code-PPP/RISS had the worst performance. Also, among the three position components, the altitude showed the worst accuracy compared to the latitude and longitude in all solutions. This was expected because the RISS is missing the z-accelerometer so it has no measure for vertical displacement. This was clear when the travel experienced sudden change of altitude (during sharp uphill and downhill) as depicted in Figures 5.10 and 5.11.

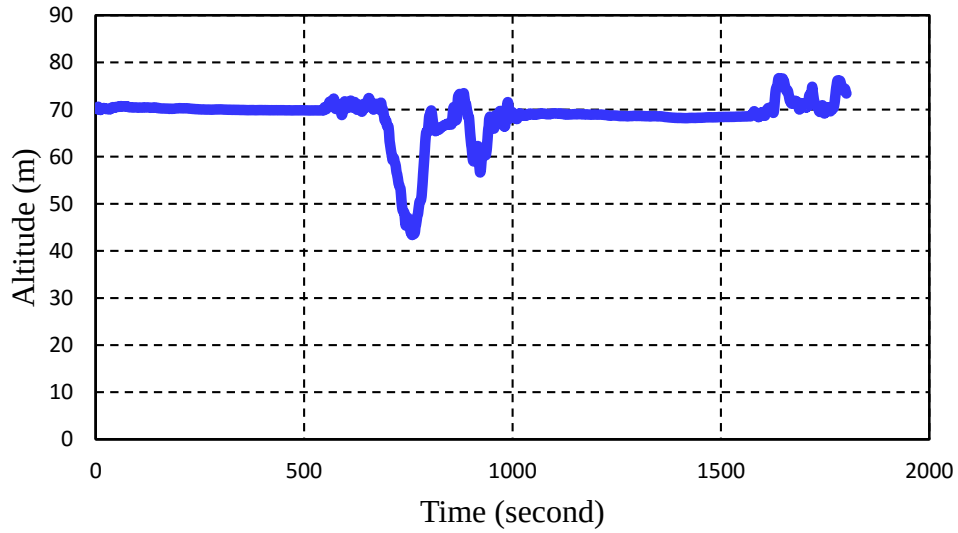

Figure 5.10 Altitude for trajectory 1

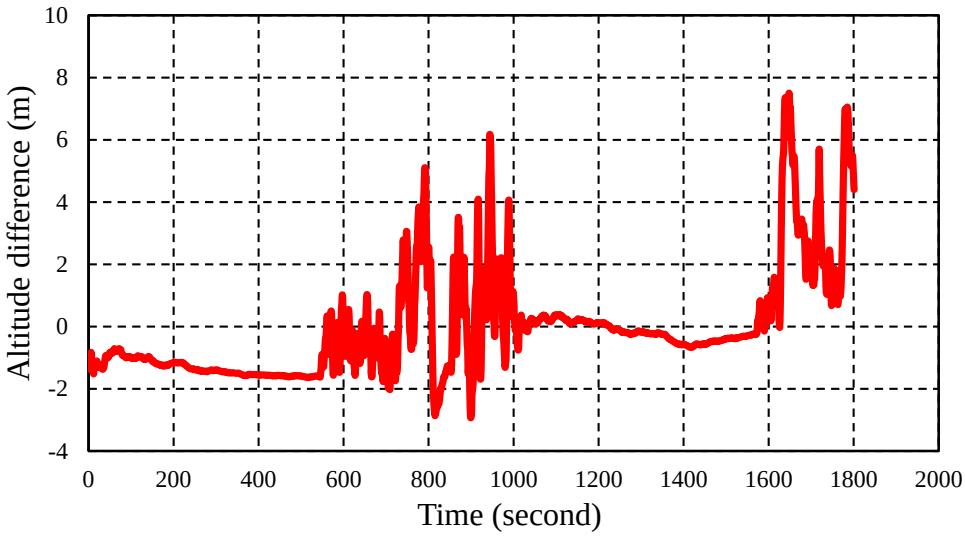

Figure 5.11 DF-code-PPP/RISS Altitude difference from reference

According to RMS error values of the trajectory for two-dimensional positioning, as depicted in Figure 5.12, code and carrier EKF-PPP/RISS performance was 27% and 11% better than EKF-SF_PPP/RISS and code EKF-PPP/RISS, respectively. For altitude, this improvement is 44% and 14% respectively during the complete trajectory.

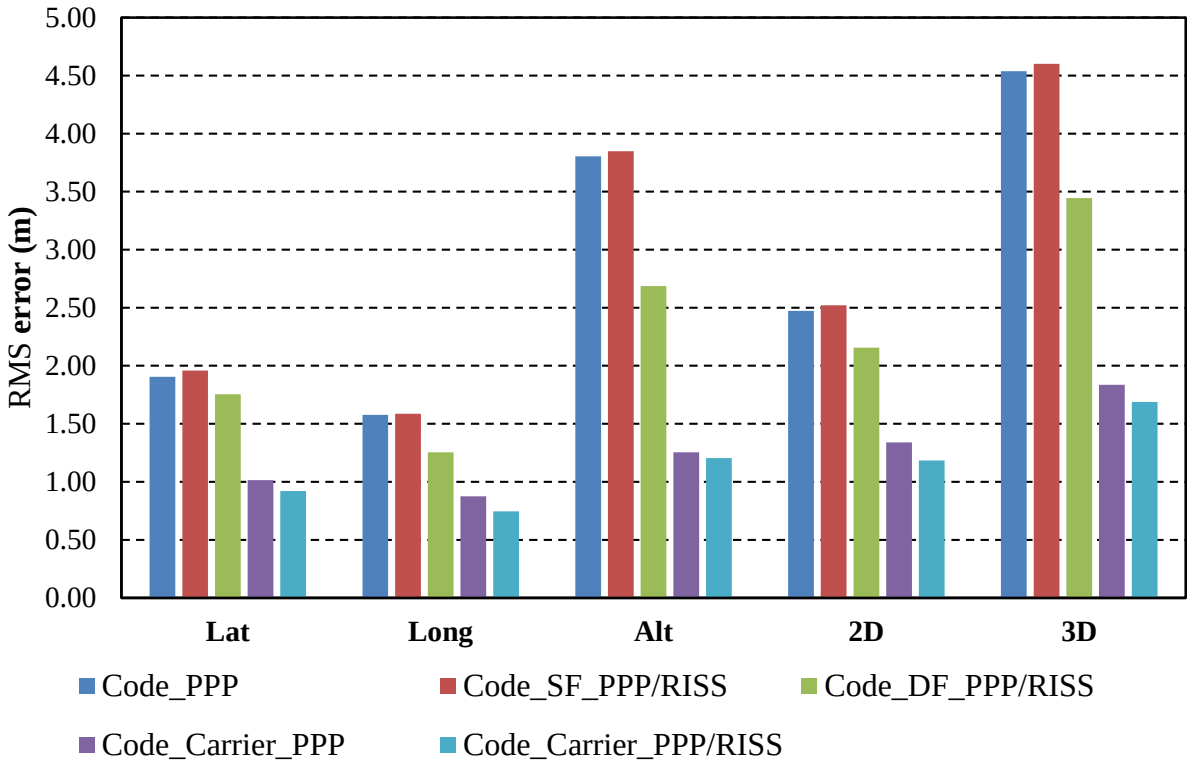

Figure 5.12 Comparison of different PPP based position solutions

In Figure 5.13 we compared smoothed-code PPP and code- and carrier-PPP. As expected, code and carrier outperformed smoothed-code solutions; however, smoothed-code solutions showed superiority over code and carrier in longitude.

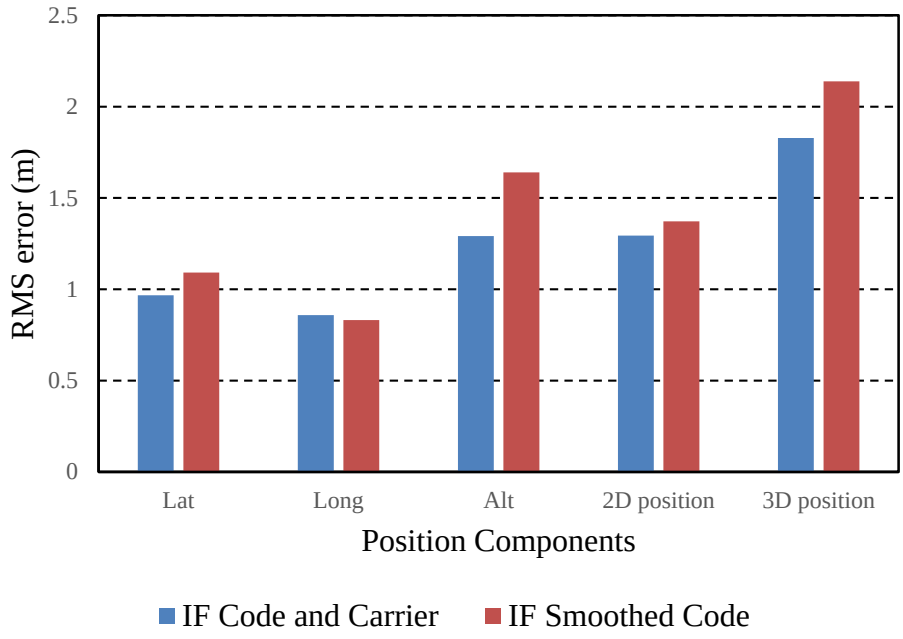

Figure 5.13 Position RMS for smoothed code and code and carrier PPP/RISS solutions

Solution convergence was measured as the time needed to converge the unchanged elements of the model state. From the 16 elements of the vector state, these unchanged elements during all trajectories are the gyroscope bias, as well as X and Y accelerometer biases. Figures 5.14, 5.15, and 5.16 depict the convergence of gyroscope bias, as well as X and Y accelerometer biases, respectively. Results showed that gyroscope bias, as well as X and Y accelerometer biases, needed 250, 60, and 150 seconds to converge, respectively.

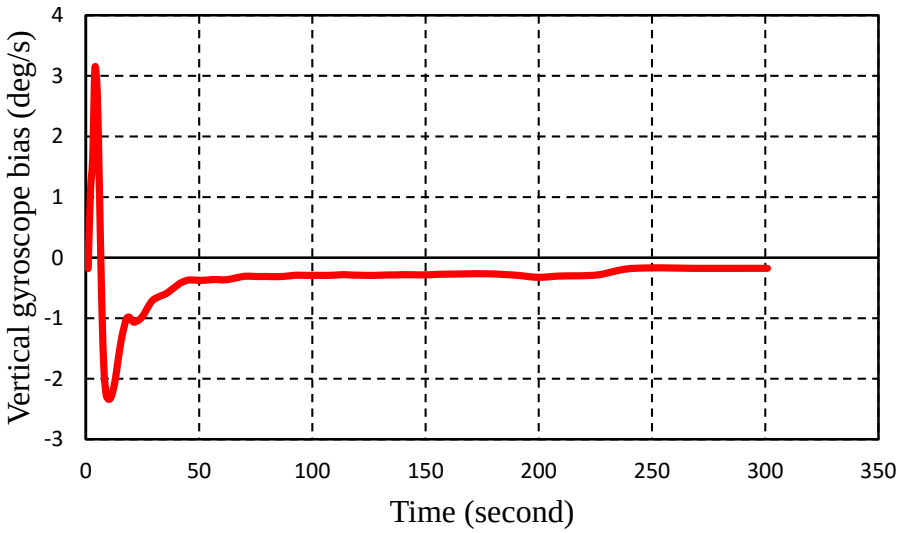

Figure 5.14 Convergence of Gyroscope Bias

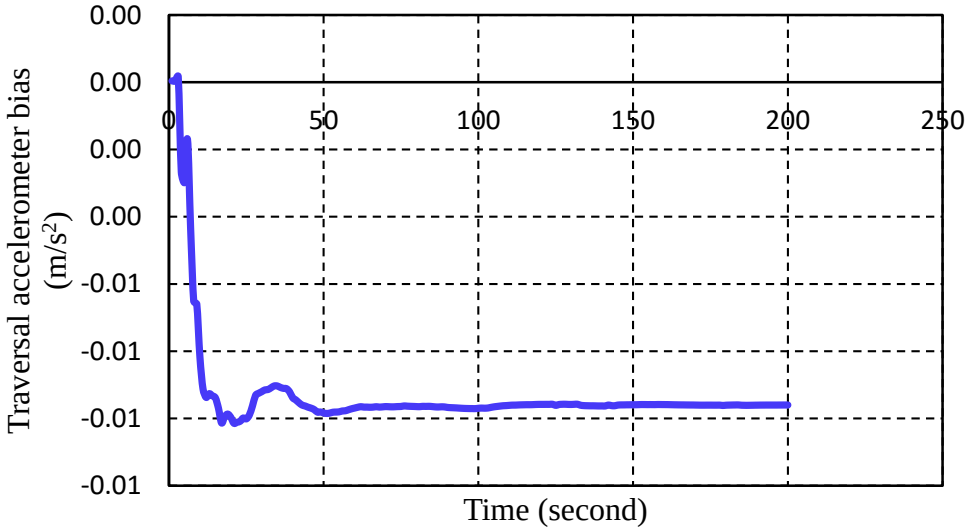

Figure 5.15 Convergence of X Accelerometer

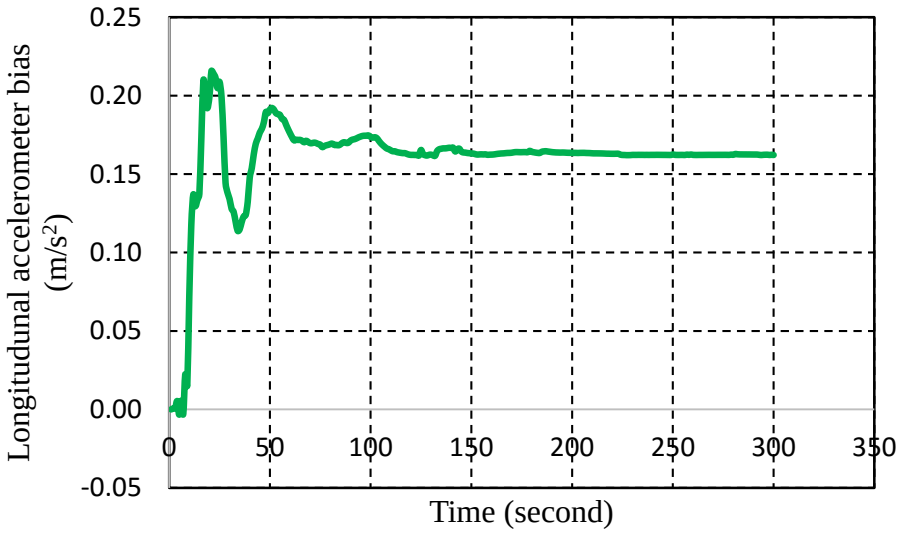

Figure 5.16 Convergence of Y Accelerometer

5.6.1 Performance during GPS signal outages

During this trajectory there was no natural GPS signal blockage, because the trajectory was planned to have continuous GPS availability in order to help generate a reference solution to evaluate the developed PPP/RISS algorithms. However, to test the PPP/RISS model, five simulated GPS signal outages were introduced in the GPS measurements at different locations along the trajectory. These GPS outages range from complete outage, with no GPS satellites available, to partial outages with one, two, or three GPS satellites available. Figure 5.17 shows the average maximum positioning error of five simulated GPS signal outages for satellite availability of zero, one, two, and three satellites. The comparison of SF-code-PPP/RISS, DF-code-PPP/RISS, SDF-code-PPP/RISS, and code-carrier-PPP/RISS algorithms show that the SF-code-PPP/RISS produced the biggest error in all GPS outage levels. On the other hand, code-carrier-PPP/RISS showed the best accuracy with maximum error of about 3.5 m. The availability of more satellite

benefits SDF-code-PPP/RISS and code-carrier-PPP/RISS more than the other models. The availability of three satellites has a significant effect on all models. Overall the maximum error was limited by 9.8 m for two or less than two satellites, and the solution depends mainly on the RISS.

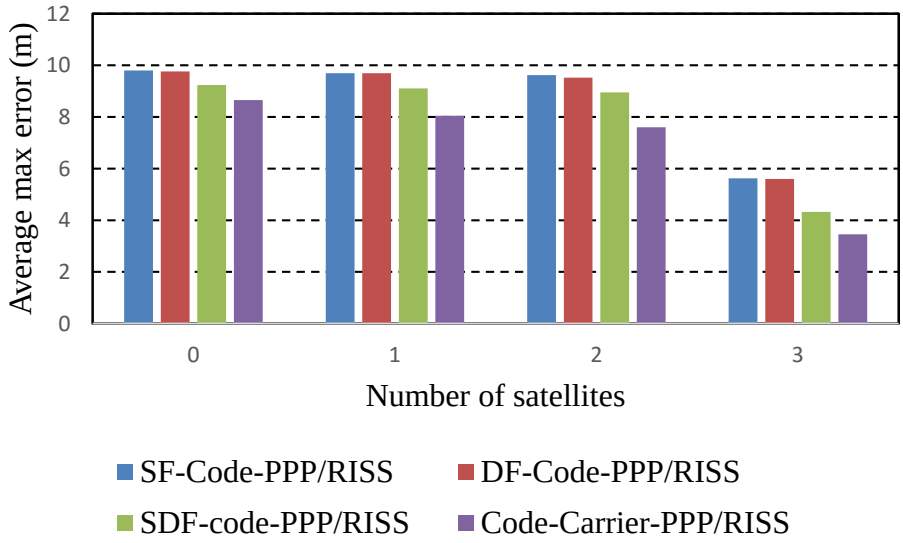

Figure 5.17 Average maximum positioning error for all models during outage of 60 seconds

Figure 5.18 shows the average maximum positioning error of five simulated GPS signal outages each of 60 second duration. The comparison of SF-code-PPP/RISS, DF-code-PPP/RISS, SDF-code-PPP/RISS, and code-carrier-PPP/RISS algorithms show that the SF-code-PPP/RISS produced the biggest error in all GPS outage levels. On the other hand, code-carrier-PPP/RISS is superior to other counterparts with a maximum error of about 4.7, 1.1, 1.2, 2.1, and 3.0 m for outages number 1, 2, 3, 4, and 5 respectively.

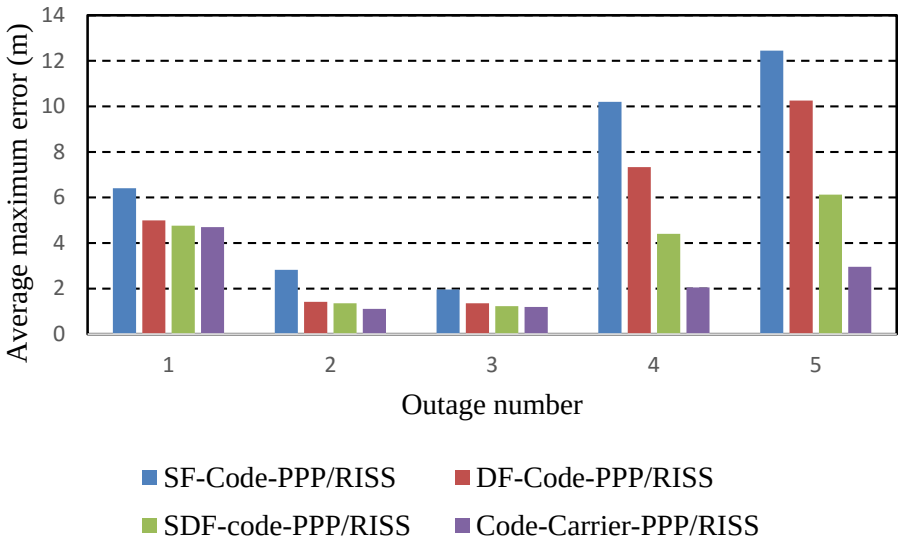

Figure 5.18 Performance of algorithms during complete GPS outage

5.7 Trajectory 2

The second trajectory was also done in Kingston, Ontario. The nature of this trajectory is an urban/suburban environment that includes the centre of the city. This road test was performed for a duration of about 50 minutes, during which a distance of 45 km was travelled while in continuous navigation. Figure 5.19 shows the route and location of trajectory 2. As with Trajectory 1, the main check for the developed system's accuracy was during GPS signal outage, intentionally introduced for a number of outages during the trajectory. Figure 5.19 also shows the locations of the outages in this trajectory, which almost covers a full level of the dynamics that could be encountered using this navigation equipment. Outages were selected to mimic conditions of a typical trip, such as during turns, straight portions, slopes, stops, and at different levels of speed.

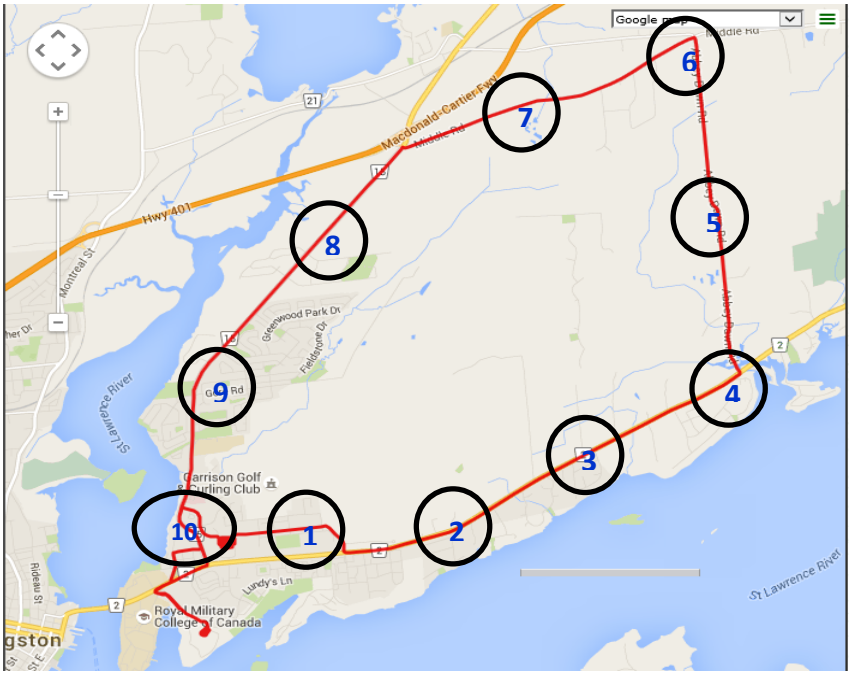

Figure 5.19 Trajectory 2

The second trajectory also included in its route suburban roads (60 – 80 km per hr. speed) as well as a highway (100 km per hr. speed). Fig 5.20 depicts the change in the vehicle speed during the period of the trajectory.

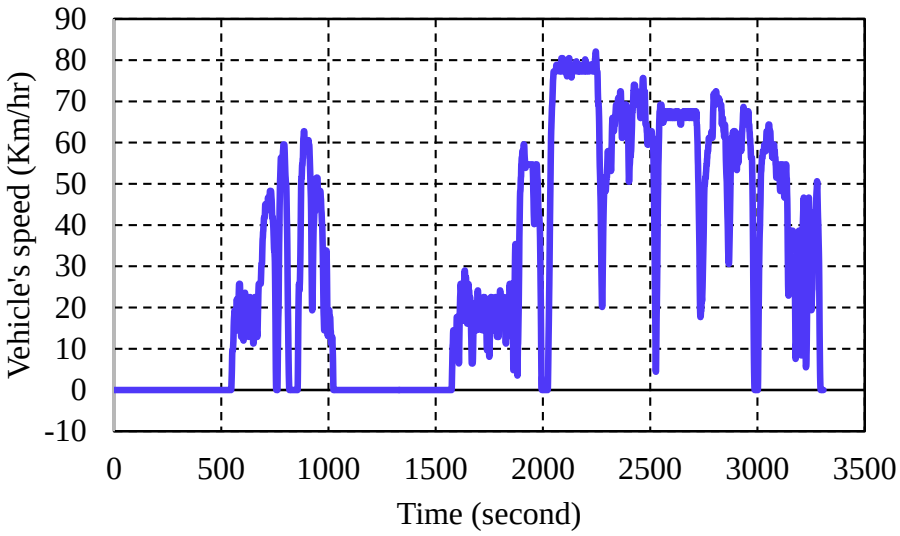

Figure 5.20 Vehicle Speed during Trajectory 2

5.7.1 Data Pre-processing

Similarly to Trajectory 1, the data pre-processing was conducted by processing the GPS observations and navigation data through TEQC software, appendix C.2 includes a detailed quality check report for the GPS data for this trajectory. Data preprocessing also included examining GDOP for the whole trajectory, multipath, and signal-to-noise ratio for each satellite. Figure 5.21 shows number of available satellites at each epoch during this trajectory. Figure 5.22 shows DOP

values. Figures 5.23 and 5.24 show sky views for multipath MP1 and MP2. Figures 5.25 and 5.26 show sky views for the SNR on L1 and L2, respectively.

For this trajectory, GDOP values indicate relatively good satellite geometry during the first half of the trajectory, while the second half shows completely bad satellite geometry. As shown in Fig 5.18, the GDOP reached a maximum of 4 in a few epochs in the first half, and in the second half, GDOP showed severe sudden changes with a maximum of 7.

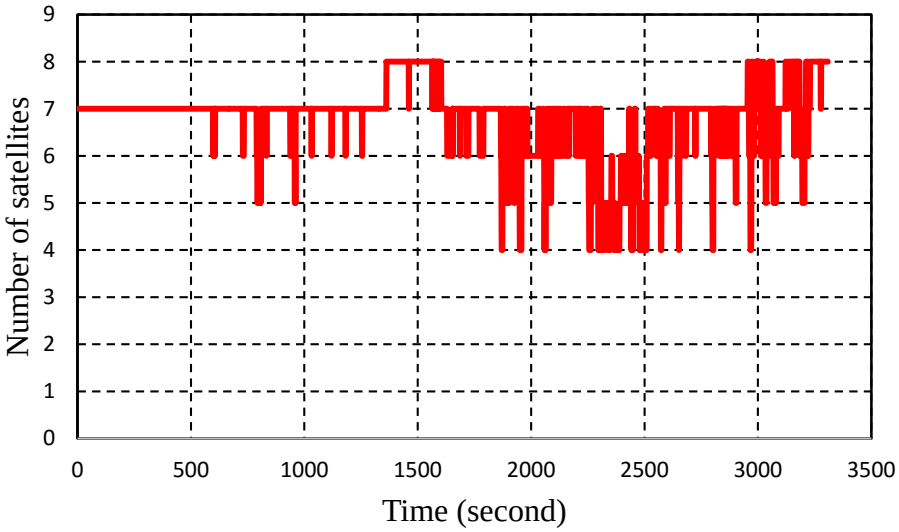

Figure 5.21 Satellites availability for trajectory 2

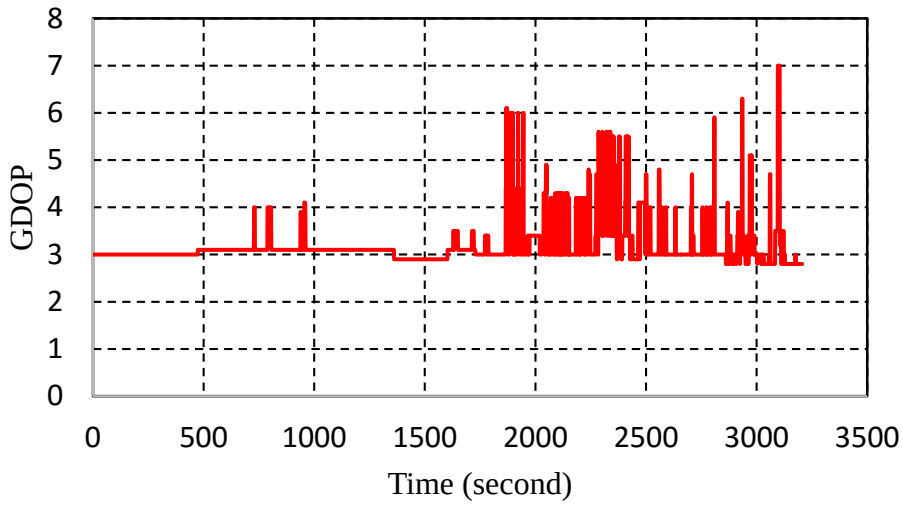

Figure 5.22 GDOP Plot for Trajectory 2

The multipath plot in Figures 5.23 and 5.24 show low multipath except for satellite G02, which shows high severity for almost half of the trajectory duration for both MP1 and MP2. Also, satellites G10 and G08 have multipath of a number of epochs. These three satellites were in low elevations, which made them more exposed to multipath. The sky plots also show the bad geometry of the satellites during this trajectory.

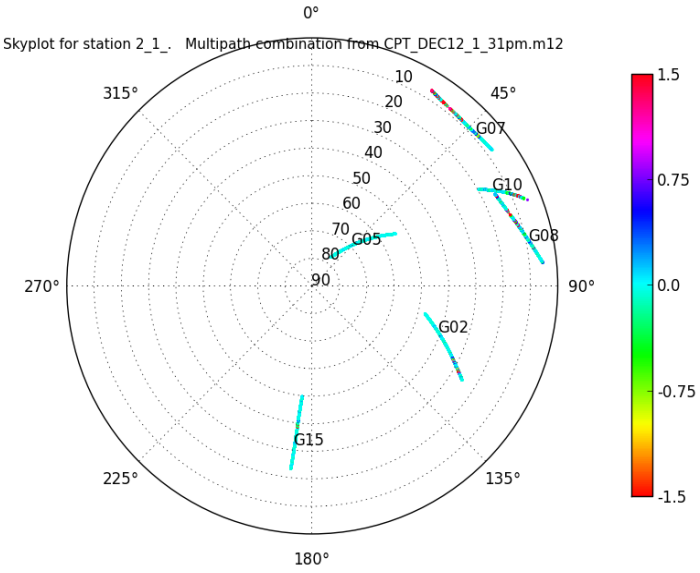

Figure 5.23 Sky view to show multipath MP12 during Trajectory 2

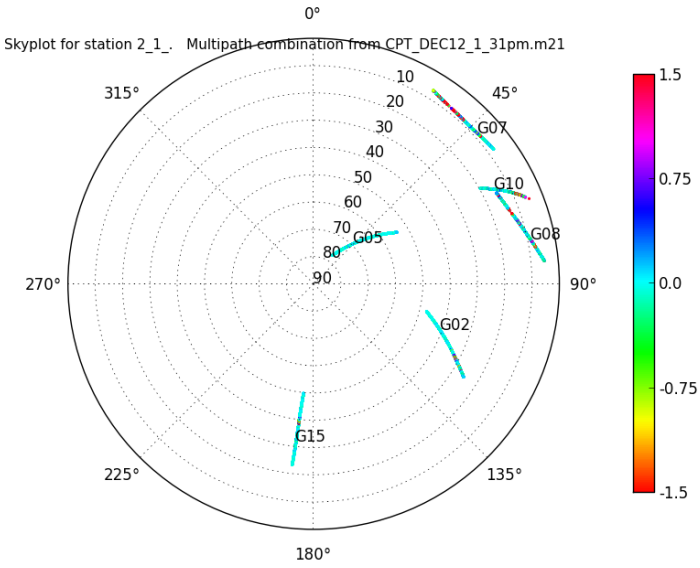

Figure 5.24 Sky view to show multipath MP21 during Trajectory 2

Following the pre-processing of Trajectory 2 data, Figures 5.25 and 5.26 show L1 and L2 SNR, respectively. L1 SNR show relatively good signal strength and agrees with the SNR in Trajectory 1, in that L1 SNR is better that L2 SNR. Again, the variability of SNR is because of the high dynamic experienced by the GPS receiver and also the high rate of data collection.

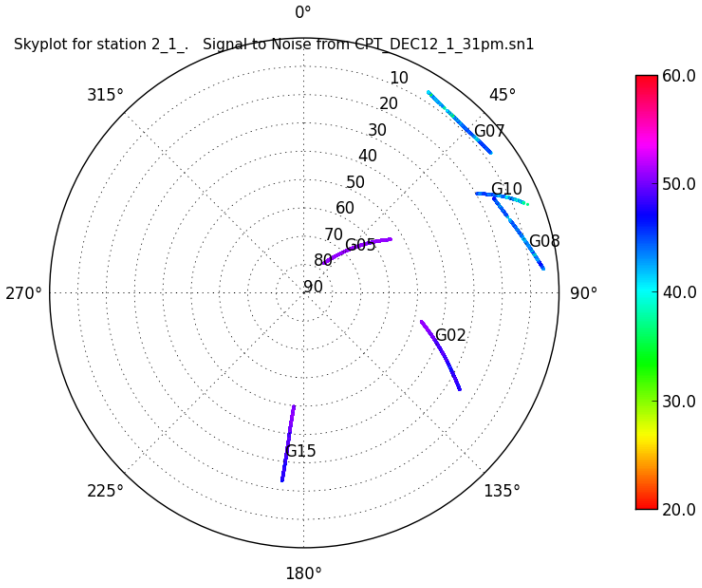

Figure 5.25 Sky view to show L1 SNR during Trajectory 2

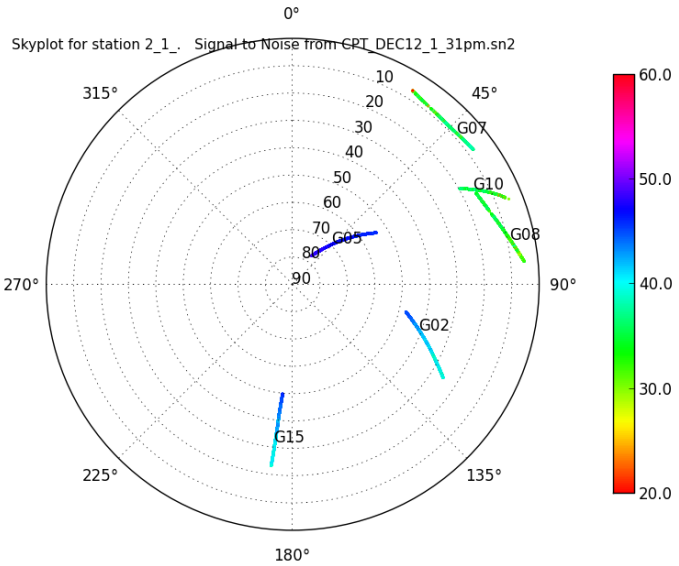

Figure 5.26 Sky view to show L2 SNR during Trajectory 2

5.7.2 Performance evaluation

The performance of the developed algorithm was tested during the whole trajectory against a reference-integrated DGPS/INS solution. In addition, we also compared solutions from the developed algorithm to code-PPP and code- and carrier-PPP solutions. Evaluation of all the three algorithm's positional accuracies was tested by means of the computed RMS errors for latitude, longitude, altitude, and two-dimensional and three-dimensional positions, which is shown in Figure 5.29. All the cases code and carrier EKF-PPP/RISS had the best performance and EKF-SFPPP/RISS had the worst performance.

According to RMS, the value of the trajectory for two-dimensional positioning as depicted in Fig 5.11 was as follows: code- and carrier-EKF-PPP/RISS performance was 27% and was 11% better than EKF-SF_PPP/RISS and code EKF-PPP/RISS, respectively. For altitude, this

improvement was 44% and 14%, respectively, during the complete trajectory. Similar to what we noticed in trajectory 1, the altitude showed the worst accuracy compared to latitude and longitude in all PPP/RISS solutions.

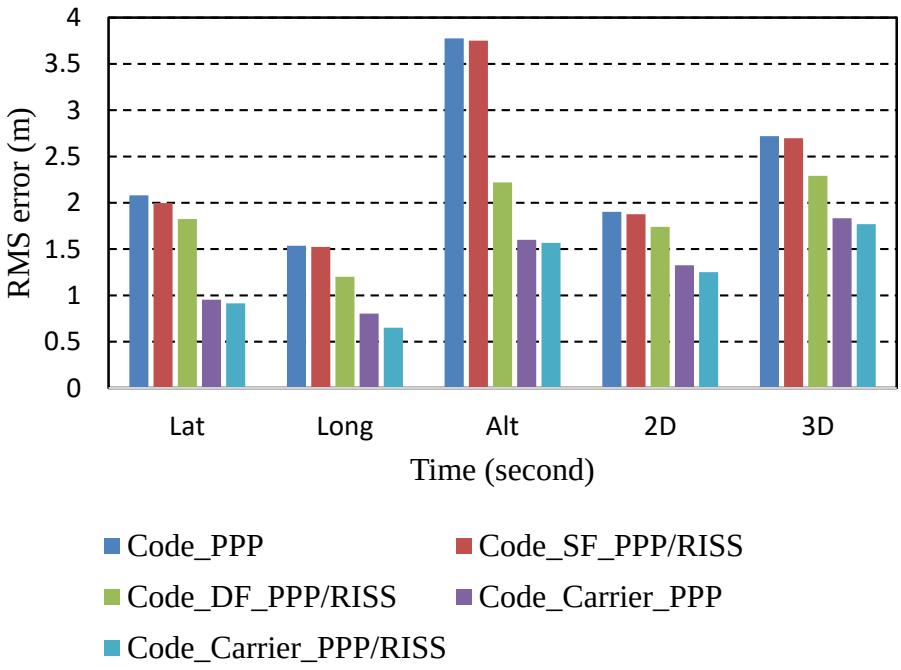

Figure 5.27 Comparison of different PPP-based position solutions

To highlight the contribution of the RISS to PPP. In figures 5.28 and 5.29 a comparison between the PPP only and PPP/RISS solutions is given. Figure 5.28 shows code-based solutions and Figure 5.29 shows code and carrier based solution. Both figure show clearly how the solution improved when PPP is integrated to RISS.

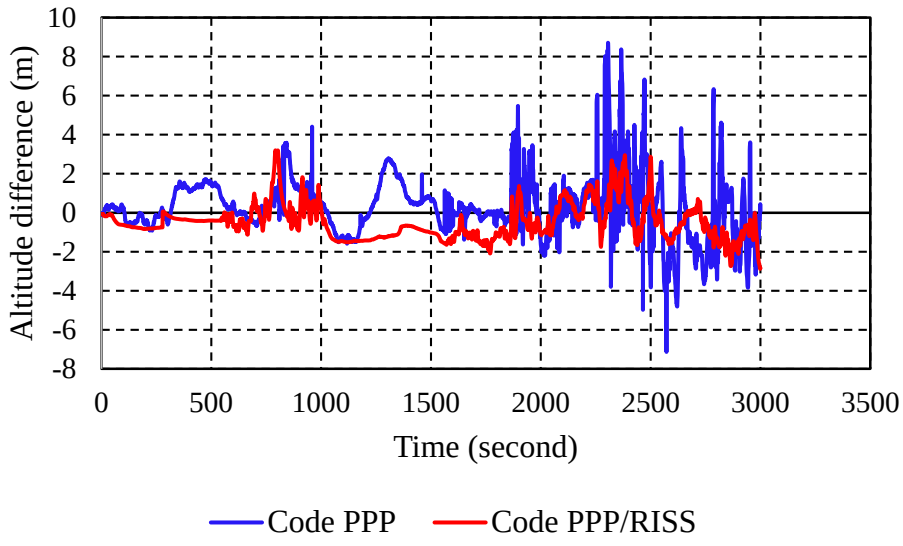

Figure 5.28 DF-Code PPP and DF-Code PPP/RISS altitude differences from reference

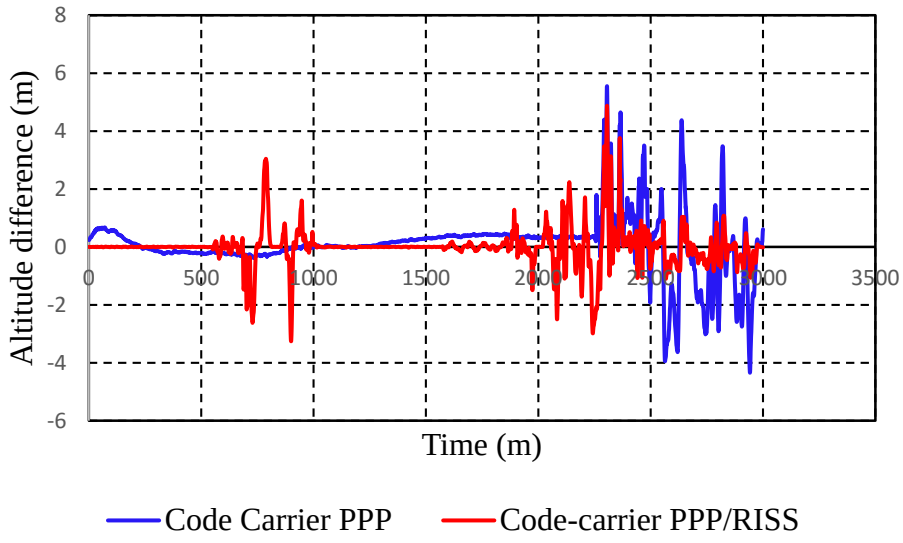

Figure 5.29 Code-carrier-PPP and Code-Carrier PPP/RISS altitude differences from reference

The solution convergence was measured as the time needed to converge the unchanged elements of the model state. From the 16 elements of the model state, the unchanged elements during all trajectories were the gyroscope bias, as well as X and Y accelerometer biases. Figures 5.30, 5.31, and 5.32 depict the convergence of gyroscope bias, as well as X and Y accelerometer biases, respectively. Results for this trajectory show that gyroscope bias, X accelerometer bias, and Y accelerometer bias converged in 400, 8, and 5 seconds, respectively.

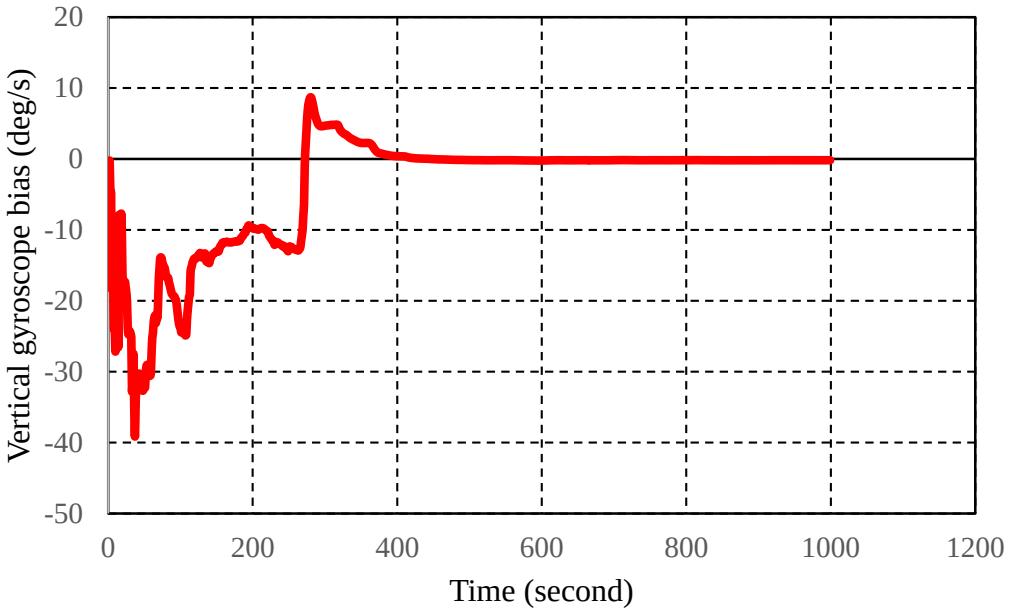

Figure 5.30 Convergence of Gyroscope Bias

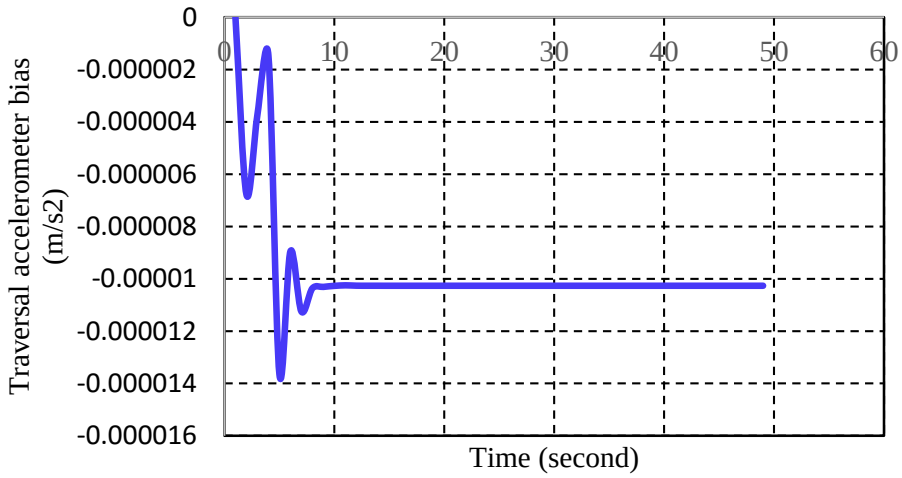

Figure 5.31 Convergence of X Accelerometer

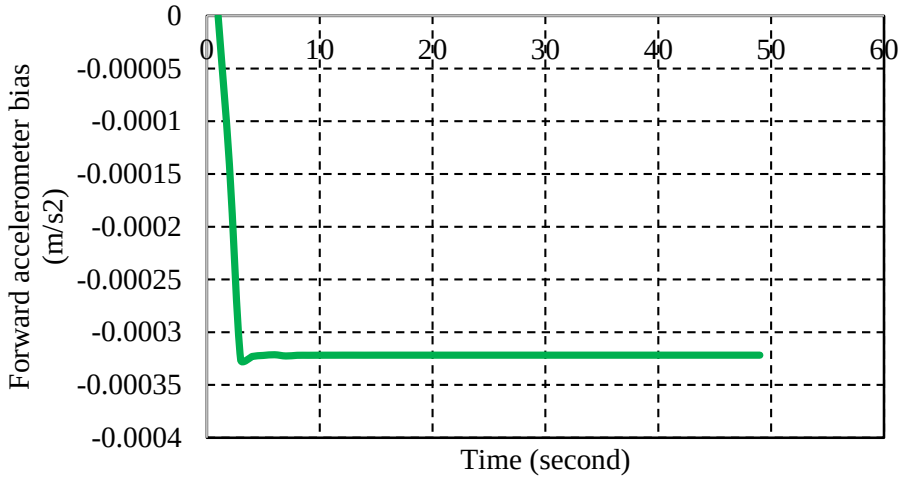

Figure 5.32 Convergence of Y Accelerometer

5.7.3 Performance during outages

Similar to Trajectory 1, during this trajectory there was no natural GPS signal blockage; this was also meant to help generate a reference solution for evaluating the develped PPP/RISS algorithms.

However, to test the PPP/RISS model for this trajectory, ten simulated GPS signal outages were introduced in the GPS measurements at different locations along the trajectory. These GPS outages ranged from complete outage, with no GPS satellites available, to partial outages with one, two, or three GPS satellites available. Figure 5.33 shows the average maximum positioning error of five simulated GPS signal outages for satellite availabilities of zero, one, two, and three satellites. The comparison of SF-code-PPP/RISS, DF-code-PPP/RISS, SDF-code-PPP/RISS, and code-carrier-PPP/RISS algorithms show that the SF-code-PPP/RISS produces the biggest error in all GPS outage levels. On the other hand, code-carrier-PPP/RISS shows the best accuracy with a maximum error of about 5.9 m. The availability of more satellite benefits SDF-code-PPP/RISS, and code-carrier-PPP/RISS more than the other models. The availability of three satellites has a significant effect on all models. Overall, the maximum error was limited by 9.8 m for two and less than two satellites, and the solution depends mainly on the RISS.

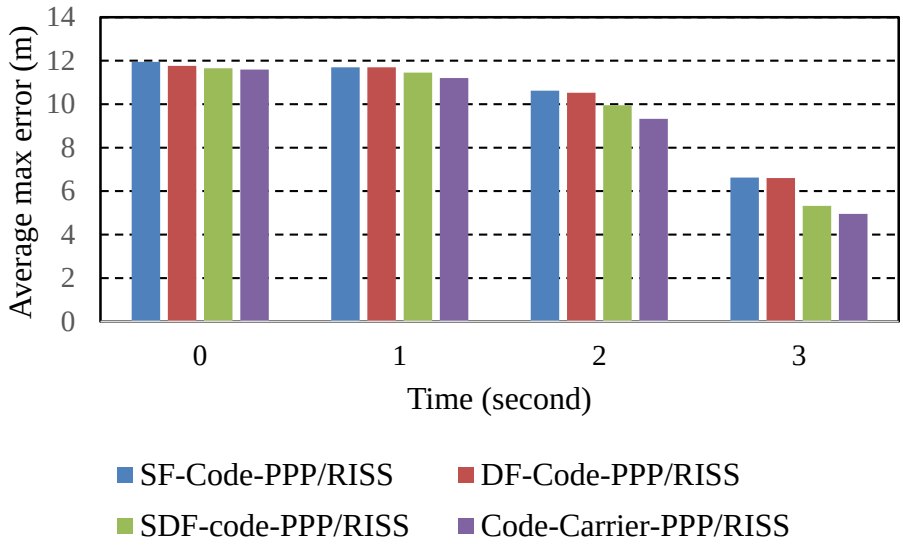

Figure 5.33 Average maximum positioning error for all models during outage of 60 seconds

Figure 5.34 shows the average maximum positioning error of five simulated GPS signal outages each of 60 second duration. The comparison of SF-code-PPP/RISS, DF-code-PPP/RISS, SDF-code-PPP/RISS, and code-carrier-PPP/RISS algorithms show that the SF-code-PPP/RISS produced the biggest error in all GPS outage levels. On the other hand, code-carrier-PPP/RISS is superior to other counterparts with a maximum error of about 3.4, 1.1, 1.2, 4.0, 4.0, 3.7, 1.9, 1.4, 4.9, and 6.0 m for outages number 1 to 10 respectively.

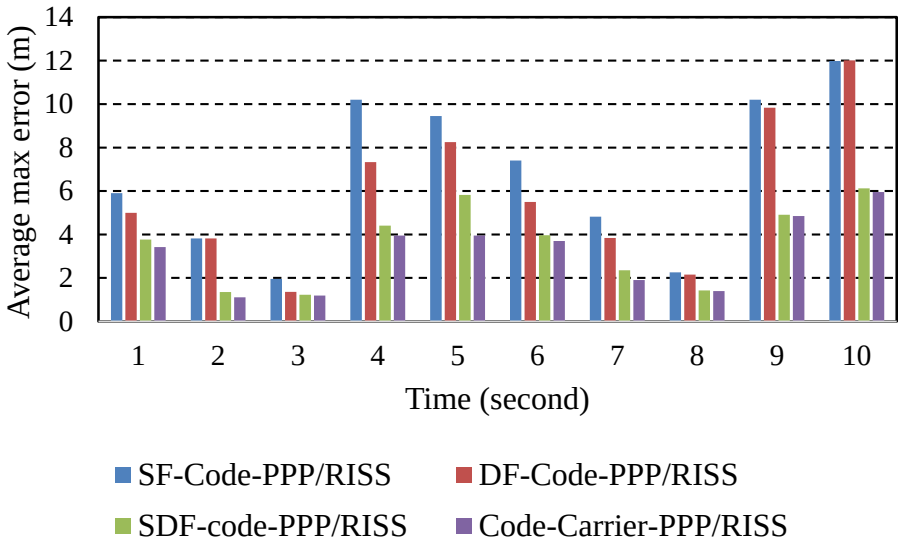

Figure 5.34 Performance of algorithms during complete outage

5.8 Trajectory 3

The third trajectory was carried out in Toronto, Ontario, Canada. Toronto is one of the largest and busiest cities in North America, having a downtown with small blocks and narrow roads, in addition to a large amount of high-rise buildings. For these reasons, this trajectory was very challenging. We were expecting to face frequent GPS signal blockage, low satellite availability, harsh multipath sharp turns, and overpass bridges. The total travelled distance in this trajectory was not long compared to about 1.5 hours of travel time. This was because of slow traffic at the time of the trajectory. Figure 5.35 shows the route for this trajectory, which includes the Don Valley Parkway (DVP) highway, Gardiner Express highway, and the busy urban routes of Spadina Street, Bloor Street, and Bay Street. Figure 5.36 depicts vehicle's speed.

Figure 5.35 Trajectory 3

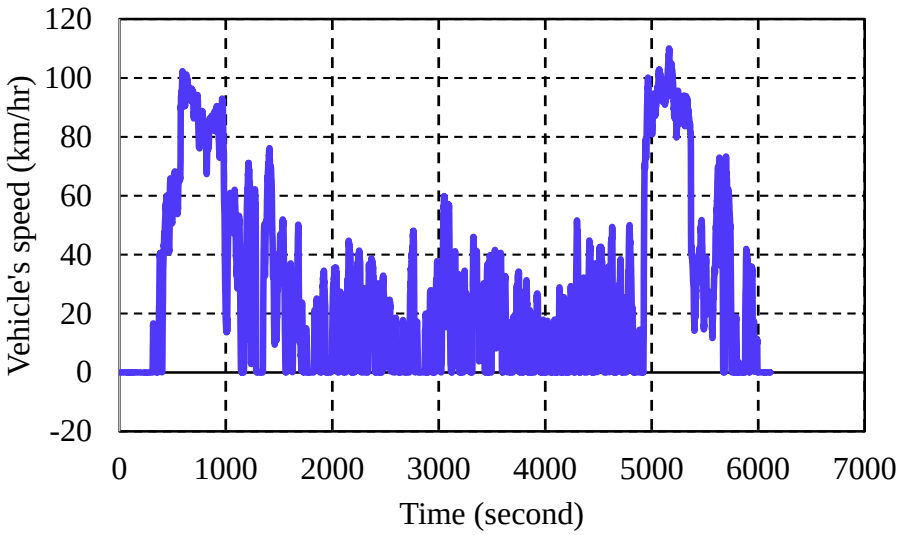

Figure 5.36 Vehicle's speed for trajectory 3

This trajectory show a real challenge for GPS and this is due to signal loss from time to time. Quality check report in appendix C.3 shows detailed analysis of the GPS observations for this trajectory. Figure 5.37 depicts the satellite availability, which shows very low satellites availability with zero satellites for varying periods of time, which creates natural GPS signal outages.

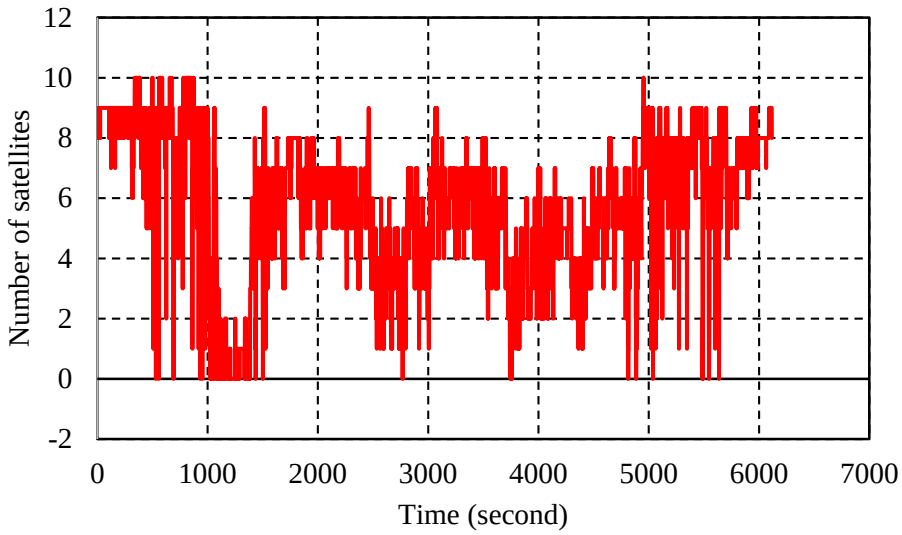

Figure 5.37 Satellite Availability during Trajectory 3

In addition to satellite availability, Figure 5.38 depicts the GDOP values, which show high GDOP in the majority of the trajectory. This indicate how challenging this trajectory for PPP only. Figure 5.39 shows how the two-dimensional positioning PPP solution is affected by frequent GPS signal loss and severe multipath.

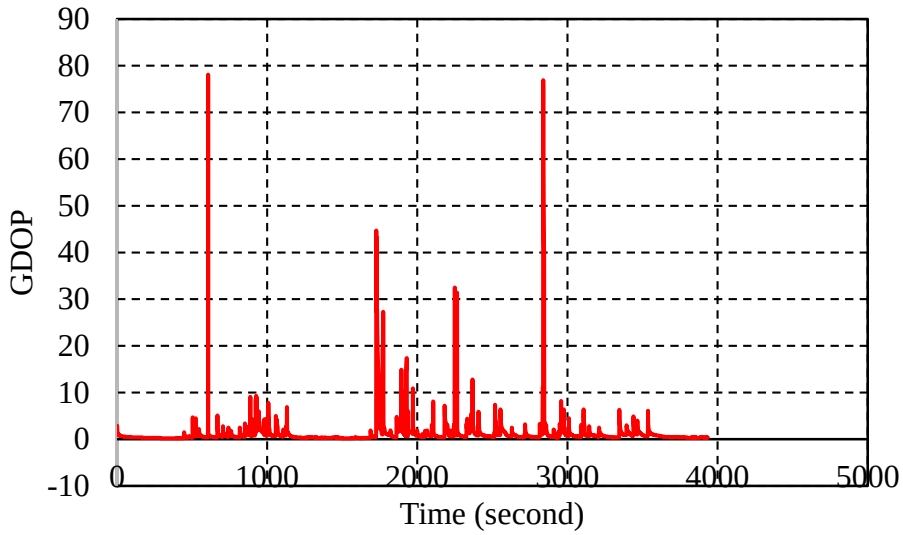

Figure 5.38 GDOP for Trajectory 3

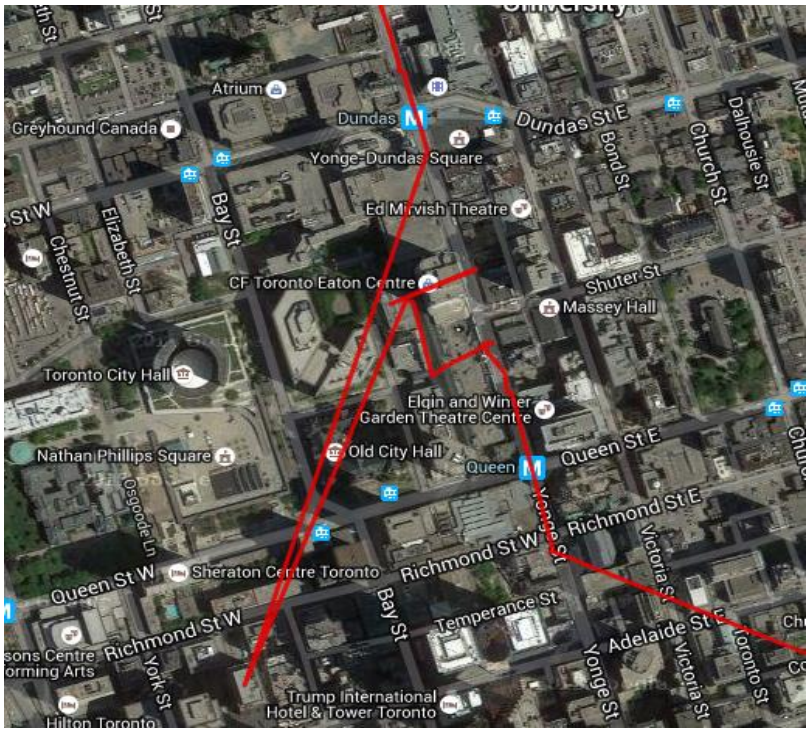

Figure 5.39 Repeated GPS signal outages

5.8.1 Performance evaluation

The performance of the developed PPP/RISS algorithms was tested in this trajectory during natural GPS signal outages. The positioning reference solution used here was obtained from the integrated tactical grade and GPS. Figure 5.40 shows nine natural outages as well as their time and durations. As the outages are natural, they have varying durations ranging from 18 to 450 seconds.

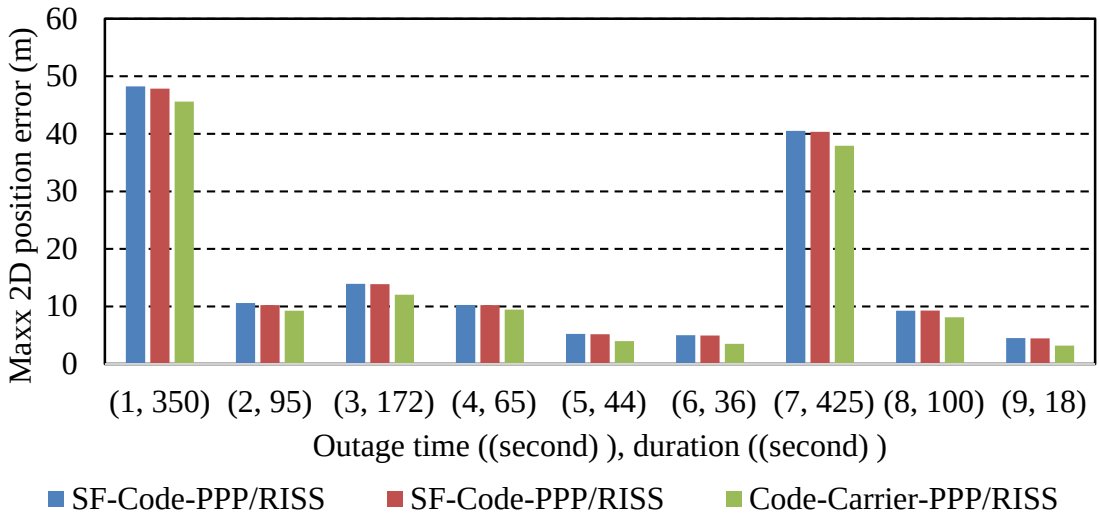

Figure 5.40 Two-dimensional position maximum error for natural outages during Trajectory 3

5.9 Summary

In this section, we summarized the positioning results from four algorithms of EKF-based tightly coupled PPP/RISS integrated systems. The test data were from three trajectories that captured different urban and suburban navigation environments. Trajectories 1 and 2 captured the same environment which is classified as urban/suburban, while trajectory 3 is classified as urban. In

summarizing the results we separate between trajectory 1 and 2 in one group (urban/suburban) and trajectory 3 in another group (urban). The results show RMS values and their averages for all trajectories for latitude, longitude, and altitude, which are summarized in Table 5.1 for group 1. In this group, Trajectory 2 showed better accuracy in all models and for most three-dimensional positioning components, although Trajectory 1 shared the same navigation environment as Trajectory 2 and also had a shorter duration and shorter travelled distance — so was less exposed to critical driving conditions. However, it showed larger RMS values than Trajectory 2 during most of the route. This is because the reference tactical grade used in Trajectory 2 has less gyroscope accuracy.

The models results show that code-carrier-PPP/RISS was superior to SF- and DF-code-PP/RISS. I latitude. Code-carrier-PPP/RISS improved the solution by 47% and 20% from SF- and DF-code-PP/RISS, respectively. In longitude, code-carrier-PPP/RISS was superior to SF- and DF-code-PP/RISS by 65% and 31%, respectively. Code-carrier-PPP/RISS continued its outperformance to SF- and DF-code-PP/RISS in altitude by 46% and 25%, respectively.

Table 5.1 Trajectories' Overall RMS (Meters) for Urban/Suburban Environment

	SF-Code-PPP/RISS				DF-Code-PPP/RISS				Code-Carrier-PPP/RISS		
	Lat	Long	Alt		Lat	Long	Alt		Lat	Long	Alt
Traj 1	1.612	1.762	2.545		1.057	0.953	1.741		0.885	0.560	1.383
Traj 2	1.720	1.580	2.014		0.992	0.780	1.804		0.905	0.641	1.257
Average	1.666	1.671	2.280		1.025	0.867	1.773		0.895	0.601	1.32

Table 5.2 summarizes metrics for the algorithms' performance during GPS signal outages. In this table the average performance for all three algorithms in situations where the satellites' availability was zero, one, two, or three satellites is presented. The GPS signal outage duration is fixed to 60 seconds for two, and zero for 60 seconds. The number of outages is five, 10 and two for Trajectories 1, 2, and 3, respectively. Taking the average of all trajectories for all satellite cases as a performance measure, we find that code-carrier-PPP/RISS outperforms SF- and DF-code-PPP/RISS by about 35% when the satellite availability level was set at three. For other satellite availability levels, all algorithms performed almost identically.

Table 5.2 Summary of 2D position average maximum error (m) at different levels of satellites availability (Urban/Suburban)

	SF-Code-PPP/RISS				DF-Code-PPP/RISS				Code-Carrier-PPP/RISS			
Satellite #	0	1	2	3	0	1	2	3	0	1	2	3
Traj 1	19.6	17.62	17.27	9.22	19.3	17.52	17.36	9.15	20.19	18.88	16.5	6.91
Traj 2	14.09	15.80	14.14	6.77	13.95	15.75	14.25	6.68	10.52	10.51	16.55	4.01
Average	16.71	15.71	8.00	16.63	16.64	15.81	15.81	7.92	15.36	14.7	16.33	5.46

Table 5.3 presents the performance of developed algorithms at different levels of satellites availability during trajectory 3.

Table 5.3 Summary of 2D position average maximum error (m) at different levels of satellites availability (Urban)

	SF-Code-PPP/RISS				DF-Code-PPP/RISS				Code-Carrier-PPP/RISS			
Satellite #	0	1	2	3	0	1	2	3	0	1	2	3
Traj 3	16.40	17.84	16.12	15.45	16.30	16.75	16.15	15.51	14.79	14.45	14.94	12.83

It is worth mentioning here that performance during GPS signal outages depends on the estimation of gyroscope bias, which determines how algorithms are going to predict its course when limited or no help is available from GPS measurements. That is why the performance of code-carrier-PPP/RISS over the other algorithms is not as significant during very low satellite availability compared to when enough satellites are available and RMS for the whole trajectory was computed.

Chapter 6

6 Summary, Conclusions and Recommendations

This chapter summarizes the work presented in this thesis and draws some conclusions from the theoretical development and results of this research. The second section of this chapter presents some recommendations for future research.

6.1 Summary and Conclusions

Currently, land vehicle navigation relies on single-frequency stand-alone GPS, which provides limited accuracy of 10 m to 15 m and requires the GPS receiver to be locked onto at a minimum of four GPS satellites to be able to generate a navigation solution. In urban canyons, however, high-rise buildings and other structures obstruct the GPS satellite signals, which present a challenge to have useful navigation solutions using GPS only. Also, road traffic safety is targeting vehicle-to-vehicle communication, which requires accurate positioning and navigation solutions of less than 2 m (e.g., this is being done through the Cooperative Vehicle Safety System [CVSS]). In the future, reliable, accurate, and low-cost navigation systems will be apriority for land-vehicle.

There are existing technologies, which can be integrated with GPS in order to overcome GPS's low satellite availability and signal blockage. However, these technologies are costly. An alternative for expensive INS technologies is the MEMS-based inertial sensors. MEMS-based sensors offer low-cost solutions, but they produce measurements with complex errors. To adapt to

these errors, robust systems need to be designed and alternatives for removing and reducing errors' sources must be employed.

In this research, the precise point positioning (PPP) technique is integrated with a reduced inertial sensors system (RISS) — a low-cost system that uses data from reduced MEMS-based inertial sensors and vehicle odometry — to provide accurate, reliable, and inexpensive land-vehicle navigation system. The developed system is integrated in a tightly coupled mode through the use of an extended Kalman filter (EKF), which employs an improved error model for the RISS data. The developed system was tested by real trajectories' data with single-frequency code PPP/RISS (SF-code-PPP/RISS), dual-frequency code PPP (DF-code-PPP/RISS), smoothed dual-frequency code PPP (S-DF-code-PPP/RISS), and code- and carrier-Phase PPP (code-carrier-PPP/RISS). The performance of the developed PPP/RISS model was evaluated by means of RMS and maximum error of position during continuous GPS availability and during GPS signal outages.

The test data in this research was collected through three road trajectories that captured different urban and suburban navigational environments. Trajectories 1 and 2 were conducted in Kingston, Ontario, Canada and Trajectory 3 was performed in Toronto, Ontario, Canada. The developed algorithms were tested when four satellites were available and also when there was a GPS signal outage with variations between zero and three satellites available. The evaluation of the results was based on the RMS and maximum errors. Also, we evaluated individual coordinate components (latitude, longitude, and altitude) and the two- and three-dimensional positioning.

The results suggest that, during satellite availability, code-carrier-PPP/RISS is superior to SF- and DF-code-PP/RISS. In latitude, code-carrier-PPP/RISS improved the solution by 47% and 20% from SF- and DF-code-PP/RISS, respectively. In longitude, code-carrier-PPP/RISS is superior to SF- and DF-code-PP/RISS by 65%, and 31%, respectively. Code-carrier-PPP/RISS continues its outperformance to SF- and DF-code-PP/RISS in altitude by 46% and 25%, respectively. During GPS signal outages of 60 seconds in duration, we concluded the performance of the developed algorithms as: the code-carrier-PPP/RISS outperformed SF- and DF-code-PPP/RISS by about 35% when the satellite availability level was at three satellites. At all other satellite availability levels, all algorithms performed almost identically.

It should be pointed out that the system performance during GPS signal outages depends on the convergence of the gyroscope bias, which determines how algorithms are going to predict its course when limited or no GPS measurements are available. This explains why the performance of code-carrier-PPP/RISS is not as significant (close to or similar to the other algorithms) during very low satellite availability.

Overall, this thesis shows that the integration of PPP and RISS eliminates the challenge of using kinematic PPP in land-vehicle navigation. The developed system employs RISS, which is in comparison to a full INS sensors, simplified the computation process, reduced the sources of error, and, in addition, saved the cost of two gyroscopes and one accelerometer.

6.2 Recommendations

This research explored the use of: (1) single-frequency code; (2) dual-frequency code-only; (3) code and carrier-phase PPP. In (1), ionospheric delay was corrected using the US total electron content (USTEC) model developed by the National Oceanic and Atmospheric Administration (NOAA), the International GNSS Service (IGS)'s final global ionospheric maps (GIM), and the Centre for Orbit Determination in Europe (CODE). In (2) and (3), ionosphere-free linear combinations were used to correct ionospheric delay. Other GPS errors commonly corrected in (1), (2) and (3) included tropospheric delay, satellite precise clock and orbit, Sagnac, relativity, satellite hardware delay, ocean loading, and signal windup (in [3]). Future improvements could be achieved through the following:

1) In this research, multipath error is only accounted for through the GPS measurements stochastic model. In future research, other techniques to mitigate multipath could be explored.
2) In future research, models for mitigating receiver hardware delay could be adopted.
3) In this research, the PPP/RISS system was based on EKF. In future research, other estimation techniques (e.g., using a particle filter) could be adopted to check for any accuracy improvement.
4) Results show that the developed PPP/RISS system works with less than four GPS satellites, so the overall efficiency of the system could be improved by including other GNSS systems — e.g., GLONASS and Galileo.
5) In this research, the MEMS-based IMU used was the Crossbow IMU 300CC, which is considered a very low grade. The Crossbow IMU 300CC has a gyroscope bias of two

degree/s, which is very high bias. Since the market for the MEMS-based IMU indicates that its price will likely drop soon, a better accuracy MEMS IMU could be obtained for the same or less than the price of Crossbow IMU 300CC. Using a better-quality MEMS-grade is recommended for enhancing the PPP/RISS system's accuracy.

6) More data from challenging urban environment such as downtown Toronto is recommended to test the developed algorithms.
7) Algorithms to employ triple-frequency GPS data could be developed.

Appendix A

Reference Coordinates Frames

Reference frame definition is an important component of mathematical models that involve positioning and navigation. In navigation and positioning, quantities of interest — such as position, velocity, acceleration, and angular rates — must be expressed relative to a particular frame of reference. The work in this thesis employs different navigation sensors. Each of the sensors used produces results of position, velocity, and/or attitude that describe a certain reference frame. The literature defines several reference frames defined that have been used in positioning and navigation applications. Of these reference frames, five distinct frames of reference and a geodetic coordinate system are defined and used in this thesis. These reference systems include: Earth-Centred Inertial (ECI), Earth-Centred Earth-Fixed (ECEF), geodetic coordinate system, local-level frame (LLF), and body frame (BF). In the following an overview is provided for these five frames as described in Noureldin (2007), Karamat (2009), and Georgy (2010).

A.1 Earth-Centred Inertial Frame

Any reference frame that obeys Newton's laws of motion and gravity is recognized as an inertial frame of reference (Farrell, 1998). Inertial frame is either stationary in space or moves with constant velocity. Measurements produced by inertial sensors along their sensitive axis are measured relative to a certain inertial frame. For platforms navigating near the Earth, it is convenient to use the Earth-Centred Inertial Frame (ECI). The axes of the ECI frame are depicted with superscripts (X^i, Y^i, $Z^{i)}$, as in Figure A.1. According to the definition in Misra and Enge

(2006), ECI originated at the centre of Earth's mass. The Z-axis is directed towards a conventional terrestrial north pole. The X-axis lies in the equatorial plane and it points towards the vernal equinox. The Y-axis is defined such that it completes a right-handed system. For short periods of time, the previous definition of ECI is satisfactory, although the Earth does not orbit around the sun with fixed speed.

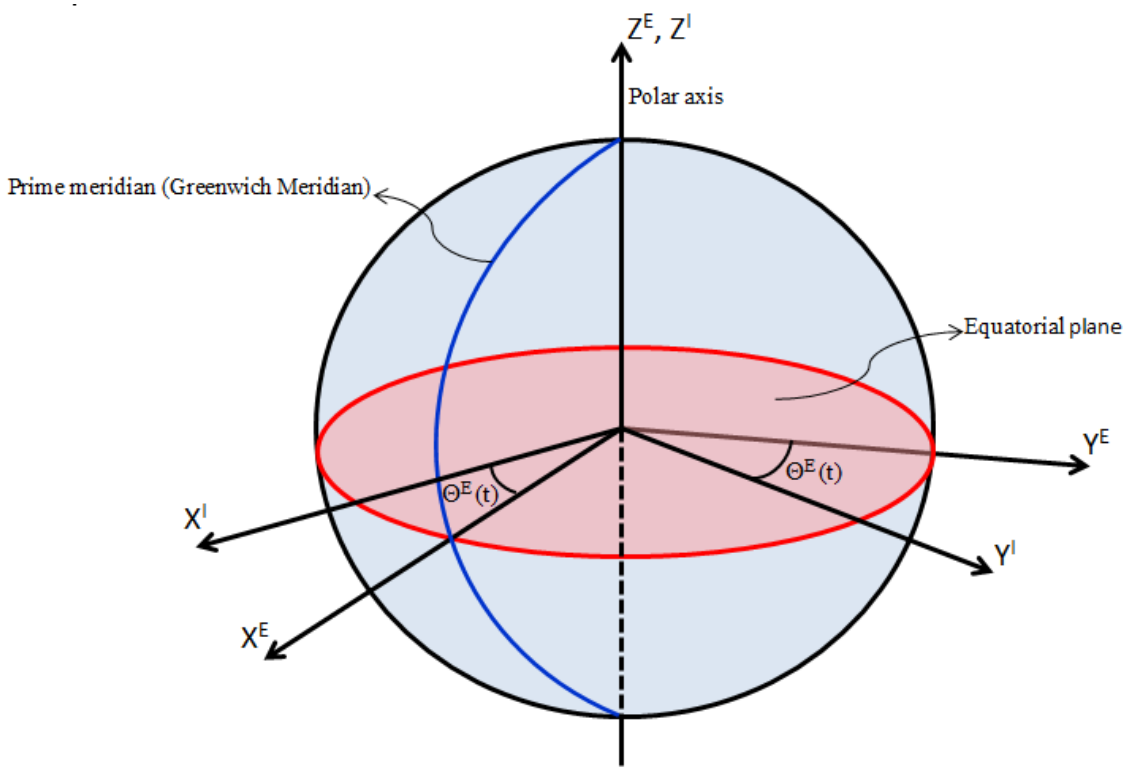

Figure A.1 Earth-Centred Inertial and Earth-Centred Earth-Fixed Reference Frames

A.2 Earth-Centred Earth-fixed Frame

ECEF is an Earth-fixed (rotating with the Earth) system of spatial Cartesian coordinates X, Y, Z. Its origin is at the Earth's centre of mass (geocentre). The three axes of ECEF are as follows. The Z-axis is directed towards a conventional terrestrial north pole. The equatorial plan is perpendicular to the Z-axis and it contains the X and Y axes. The X-axis passes through the intersection of the equatorial plane and the Greenwich meridian (prime meridian). The Y-axis is defined such as to complete a right-hand coordinate system. Figure A.1 shows an ECEF frame relative to an ECI frame.

A.3 Geodetic Coordinate System

The geodetic coordinate system is a polar representation of the ECEF reference frame. It is established by modelling the Earth with an ellipsoid in such a way that the vertical axis of the ellipsoid aligns with the ECEF Z-axis. The ellipsoid model of the Earth is used to relate geodetic latitude and longitude coordinates to ECEF coordinates. Figure A.2 shows the relationship between geodetic coordinate system and ECEF reference frame.

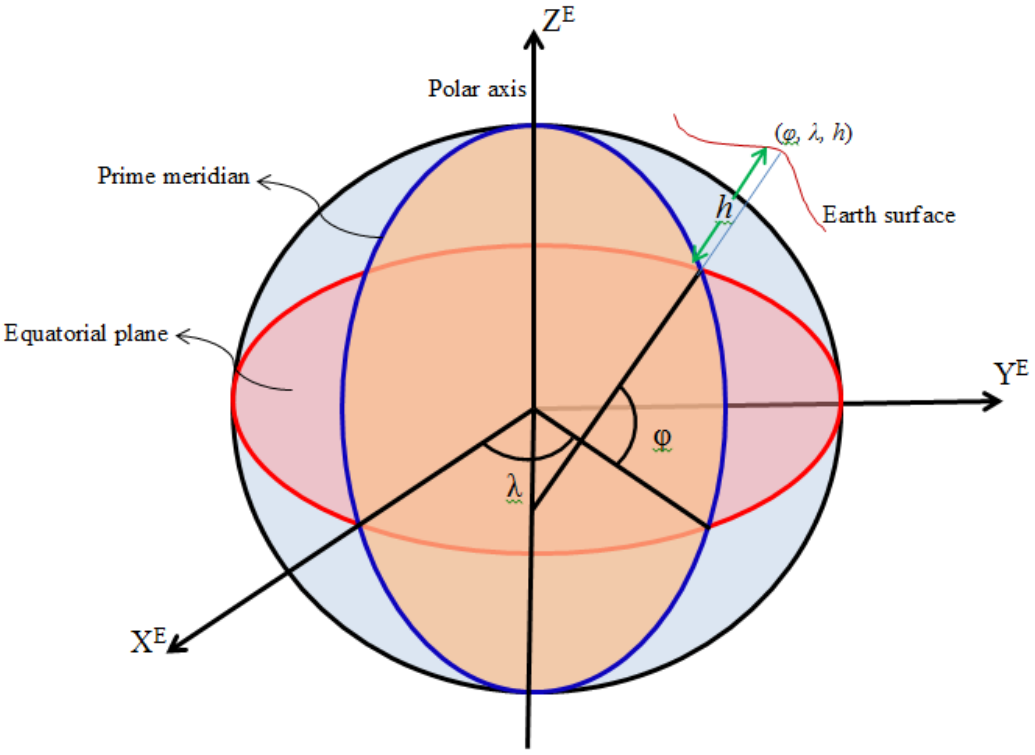

Figure A.2 Geodetic Coordinate System

A.4 Local Level Frame

A local level frame (also known as the navigation frame) originates at the centre of gravity of the moving platform. LLF is used to express the moving platform's attitude and velocity when operating on or near the Earth's surface. In this thesis, LLF axes are as follows. The X-axis points east. The Y-axis points north. The Z-axis points upward to the Earth's reference ellipsoid such as to complete a right-handed coordinate system. LLF is also referred to as ENU (east, north, and up).

There is also another commonly used version of LLF that has the X-axis pointing north, Y-axis pointing east, and Z-axis pointing down (normal to the Earth's reference ellipsoid) such as to complete a right-handed coordinate system. This frame is known as the NED frame. Figure A.3 illustrates the ENU.

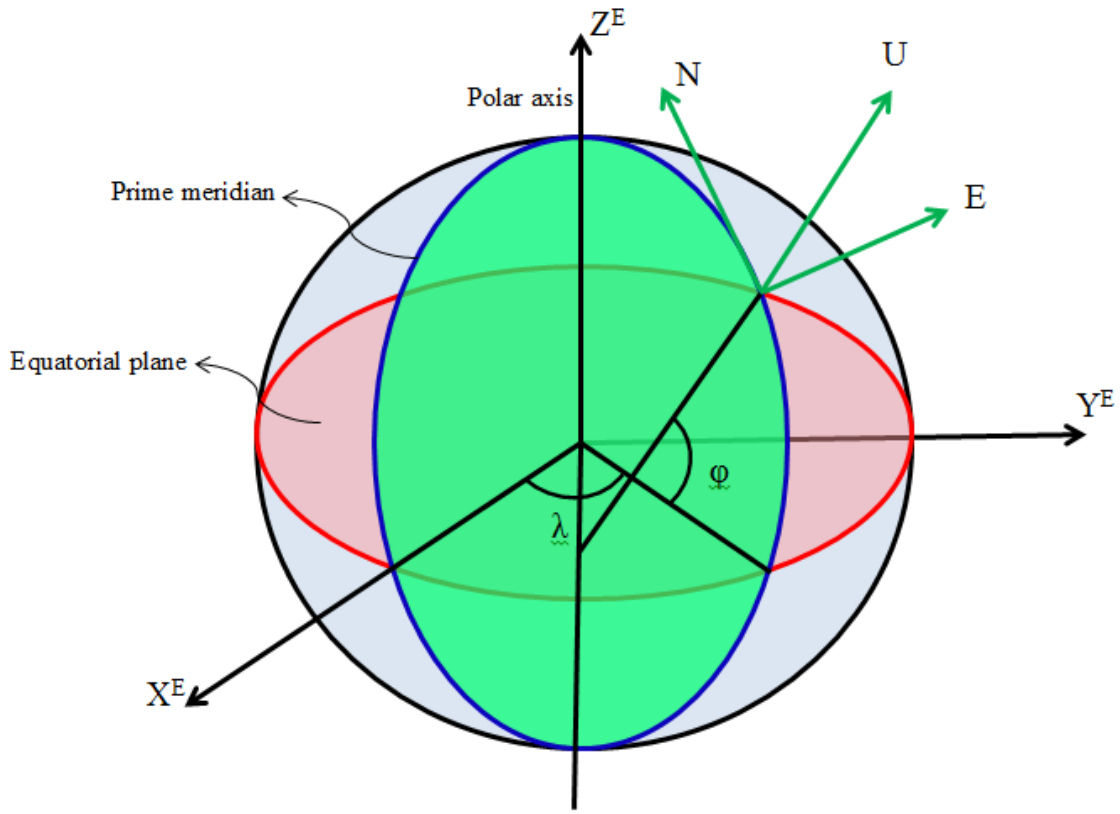

Figure A.3 North-oriented Local Level Frame

A.5 Body Frame

Body frame is the moving-platform frame. Its origin coincides with the centre of mass of the moving platform. Body frame axes are oriented as follows. The Z-axis points towards the upward

vertical direction of the moving platform. The Y-axis points towards the forward direction. The X-axis points towards the transverse direction completing a right-handed coordinate system. In land vehicle navigation applications, the axis set defined by the sensitive axes of the accelerometer sensors aligned with the axes of the moving platform in which the sensors are mounted (i.e., aligned with the body frame). The body frame used in this thesis is shown in Figure A.4.

Figure A.4 Body Frame

A.6 Coordinates Transformation

A.6.1 ECI to ECEF

ECI and ECEF have coincident origins and Z-axes. The transformation between ECI and ECEF can be implemented by applying a single rotation around the Z-axis, as follows:

$$\begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{ECEF} = R_z(\omega, \Delta t) \begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{ECI} \quad \text{(A.3)}$$

Here ω is the Earth angular velocity (ω =7.2921151467*10-5rad/sec), ($\Delta t = t - t_0$) is the difference between the epoch at time (t) and the epoch when the two frames are coincide at initial time (t_0), and $R_Z(.)$ is the rotation matrix around the Z-axis, with the angle in parentheses measured counterclockwise.

A.6.2 ECEF to ENU

ECFE and ENU have different origins and axes orientation. The transformation between ECI and ECEF can be implemented by applying rotations a single to align both frames axes and then applying a shift to unify the origins of the systems, as follows (Farwell, 1998; 2008):

$$\begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{E\mathrm{NU}} = R_x(90° - \varphi).R_z(90° + \lambda).\begin{bmatrix} \Delta X \\ \Delta Y \\ \Delta Z \end{bmatrix}_{EC\mathrm{FC}} \quad \text{(A.2)}$$

$$\begin{bmatrix} \Delta X \\ \Delta Y \\ \Delta Z \end{bmatrix} = \begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{ECEF} - \begin{bmatrix} X_0 \\ Y_0 \\ Z_0 \end{bmatrix}_{ECEF} \quad \text{(A.3)}$$

here φ and λ are the geodetic latitude and longitude of the origin of ENU, $[X_0 \quad Y_0 \quad Z_0]^T$ is the Cartesian coordinates of the origin of ENU system, $R_X(.)$ and $R_Z(.)$. The rotation matrices around the X- and Z- axis, respectively, with the angle in parenthesis measured anticlockwise.

A.6.3 Transformation of velocity from ECEF to ENU

The velocity transformation from ECFC to ENU requires the rotations of the X- and Z- axis to align the ECEF fame with ENU. The mathematical formula is given as:

$$\begin{bmatrix} \mathrm{V}_E \\ \mathrm{V}_N \\ \mathrm{V}_U \end{bmatrix} = R_x(90° - \emptyset).R_z(90° + \lambda).\begin{bmatrix} \mathrm{V}_X \\ \mathrm{V}_Y \\ \mathrm{V}_Z \end{bmatrix}_{EC\mathrm{EF}} \tag{A.4}$$

here $[\mathrm{V_E} \quad \mathrm{V_N} \quad \mathrm{V_U}]^{\mathrm{T}}$ is the velocity vector in ENU frame, and $[\mathrm{V_X} \quad \mathrm{V_Y} \quad \mathrm{V_Z}]_{\mathrm{ECEF}}$ is the velocity vector in ECEF frame. $R_X(.)$ and $R_Z(.)$ are rotation matrices around the X- , Y-, and Z- axis, respectively, with the angle in parenthesis measured counterclockwise.

A.7 ENU to Body Frame

The transformation of ENU into body frame requires three consecutive rotations. It given in mathematical representations as:

$$\begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{\mathrm{Body}} = \mathrm{R}_n^b.\begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{\mathrm{ENU}} \tag{A.5}$$

$$R_n^b = \mathrm{R_Y(r).R_X(p).R_Z(-A)} \tag{A.6}$$

$$R_n^b = \begin{bmatrix} \cos r & 0 & -\sin r \\ 0 & 1 & 0 \\ \sin r & 0 & \cos r \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 \\ 0 & \cos p & \sin p \\ 0 & -\sin p & \cos p \end{bmatrix} \begin{bmatrix} \cos -A & \sin -A & 0 \\ \sin -A & \cos -A & 0 \\ 0 & 0 & 1 \end{bmatrix} \quad \text{(A.7)}$$

$$R_n^b = \begin{bmatrix} \cos r & 0 & -\sin r \\ 0 & 1 & 0 \\ \sin r & 0 & \cos r \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 \\ 0 & \cos p & \sin p \\ 0 & -\sin p & \cos p \end{bmatrix} \begin{bmatrix} \cos -A & \sin -A & 0 \\ \sin -A & \cos -A & 0 \\ 0 & 0 & 1 \end{bmatrix} \quad \text{(A.8)}$$

$$R_n^b$$

$$= \begin{bmatrix} \cos r \cos A + \sin r \sin p \sin A & -\cos r \sin A + \sin r \sin p \cos A & -\sin r \cos p \\ \cos p \sin A & \cos p \cos A & \sin p \\ \sin r \cos A - \cos r \sin p \sin A & -\sin r \cos A - \cos r \sin p \sin A & \cos r \cos p \end{bmatrix} \quad \text{(A.9)}$$

here *r*, *p*, and *A* are the roll, pitch, and azimuth of the moving object, respectively. $R_X(.)$, $R_X(.)$ and $R_Z(.)$ are rotation matrices around the X- , Y-, and Z- axis, respectively, with the angle in parenthesis measured counterclockwise.

Appendix B

A.1 Crossbow IMU 300CC Specifications

Table B.1 Crossbow IMU 300CC Specifications

Parameter	**Specification**
Size	7.62x9.53x8.13 cm
Weight	0.5 Kg
Power consumption	<3 W
Input voltage	+9 to +30
Gyro/accelerometer technology	MEMS
Update rate	200 Hz
Start-up time	<1 S
Accelerometer Range	±2g
Accelerometer Bias	<±30 mg
Accelerometer Scale factor	<1%
Accelerometer Random Walk	<0.15m/s√hr
Gyroscope Range	±100°/s
Gyroscope Bias	20°/S
Gyroscope Scale factor	<1%
Gyroscope Random Walk	<2.25°/√hr

Table B.2 NovAtel SPAN CPT Specifications

Parameter	**Specification**
Size	15.2x16.8x8.9 cm
Weight	2.29 Kg
Power consumption	13 W
Input voltage	+9 to +3018
Gyro/accelerometer technology	Gyro: FOG; Accelerometer: MEMS
Update rate	100 Hz
Start-up time	N/A
Accelerometer Range	± 10 g
Accelerometer Bias	50 mg
Accelerometer Bias Stability	±0.75 mg
Accelerometer Scale factor	4000 ppm
Accelerometer Random Walk	N/A
Accelerometer Linearity	N/A
Gyroscope Range	±375°/s
Gyroscope Bias	20°/hr
Gyroscope Scale factor	1500 ppm
Gyroscope Random Walk	<0.0667°/√hr

Table B.3 Honeywell HG1700 IMU

Parameter	**Specification**
Size	16x16.8x10 cm
Weight	3.4 Kg
Power consumption	12 W
Input voltage	+ 12 to + 18 VDC
Gyro/accelerometer technology	Gyro: RLG; Accelerometer: RBA
Update rate	100 Hz
Start-up time	0.8 s
Accelerometer Range	± 50 g
Accelerometer Bias	<± 1 mg
Accelerometer Bias Stability	±0.75 mg
Accelerometer Scale factor	300 ppm
Accelerometer Random Walk	< 0.198 m/s√hr
Gyroscope Range	±1000°/s
Gyroscope Bias	1.0°/hr
Gyroscope Scale factor	150 ppm
Gyroscope Random Walk	<0.125°/√hr

Table B.4 SPAN Reference System

Parameter	Specification
Size	20x24.8.8x7.8 cm
Weight	3.4 Kg
Power consumption (typical)	Single antenna: 10 W; Dual antenna: 12 W
Input voltage	+ 9 to + 28 VDC
Position accuracy (RMS) Single point L1	1.50 m
Single point L1/L2	1.20 m
SBAS	0.60 m
DGPS	0.40 m
CDGPS	0.60 m
RT-20	0.20 m
RT-2	0.01 m + 1ppm
Velocity accuracy (RMS)	0.03 m/s
Attitude accuracy (RMS) Pitch	0.02°
Roll	0.02°
Azimuth	0.06°
Time accuracy (RMS) Cold Start	60 s
Hot Start	35 s
GPS Measurement Precision (RMS) L1 C/A Code	0.06 m
L2 P(Y) Code	0.08 m
DGPS L1 Carrier Phase	0.50 mm
DGPS L2 Carrier Phase	1.00 mm
Signal reaquisition	±0.75 mg
Accelerometer Scale factor	300 ppm

Accelerometer Random Walk	< 0.198 m/s√hr
Gyroscope Range	±1000°/s
Gyroscope Bias	1.0°/hr
Gyroscope Scale factor	150 ppm
Gyroscope Random Walk	<0.125°/√hr

Table B.5 GPS error Budget (Misra and Enge, 2001; Abdelsalam, 2005; Du, 2010)

Common Error Source	**Error Magnitude**
Satellite orbit	2.0 m
Satellite Clock	2.0 m
Ionospheric Delay	2.0 ~ 10.0 m at zenith
Tropospheric Delay	2.3 ~ 2.5 m at zenith
Multipath	In clean environment Code: 0.5 – 1.0 m Carrier: 0.5 – 1.0 cm
Receiver Noise	Code: 0.25 – 0.5 m Carrier: 1.0 – 2.0 mm

Appendix C

C.1 Quality check Report of GPS Observations for Trajectory 1

Version: teqc 2015Nov6

```
 SV+-----------|-----------------------|----------------------|-----------+ SV
 19|+LLL+LL_LL_LL__LLL_L+_LLLLLLLLLL_L+LL+_____________________________| 19
 27|oIooooooooooooooooooooooooooooooooooooooooooooooIooooooooooo-Looooo-ooo| 27
 21|oooooIIIII-I-II-I-IIIIIIooI--ooooooooooooo--ooooIo-oooIIoIooooo-II--oooo| 21
  9|oooooooooooooooooooooooooooooooooooooooooooooooIooooooooooooooooooooo|  9
 14|oooooooooooooooooooooooooooooooooooooooooooooooIooooooooooooIooooooooo| 14
 22|ooooooooooooooooooooooooooooooooooooooooooooooooIooooooooooooooooooooo| 22
 12|o-II-III-I--IIIII-IIIIIIoooIIooooooooooooooIooooIoooooIoo-oI-LIooo-ooooo| 12
 15|oIooooooooooooooooooooIooooooooooIooooooooooooooIoooIoIo-Io-IIIIooooIooo| 15
 18|oooooooooooooooooooooooooooooooooooooooooooooooooIooooooooooooooooooooo| 18
 25| _____________________________L+LLLL_LL+++LL+LLLLLLL+LLLLL+LLL+++| 25
S33|     L LLLL                          L                 |S33
S38|     L LL LL                         L           L     |S38
-dn|    1     1         +    +       +         +              |-dn
+dn| 21112222222222222222223   22     1      12    8 1 1 3112 2312111212   |+dn
+10|8888888888888888888888888888888888888888888888888888888888888888888888|+
10
Pos|oooo o   o     o o oo ooooooooooo ooooo                        |Pos
Clk|                                                 |Clk
   +-----------|-----------------------|----------------------|-----------+
16:25:10.000                                        16:54:29.000
2011 Apr 22                                         2011 Apr 22
```

****

QC of RINEX file(s) : propakv3_apr_22_11.11o
Input RnxNAV file(s) : propakv3_apr_22_11.11n

****

4-character ID   : PAK3
Receiver type   : NovAtel GPSCard
Antenna type   :

Time of start of window : 2011 Apr 22 16:25:10.000
Time of end of window : 2011 Apr 22 16:54:29.000
Time line window length : 29.32 minute(s), ticked every 10.0 minute(s)
  antenna WGS 84 (xyz) : 1079532.3544 -4441266.8419 4433859.4788 (m)
  antenna WGS 84 (geo) : N 44 deg 19' 20.75" W 76 deg 20' 17.31"
  antenna WGS 84 (geo) :   44.322430 deg   283.661857 deg (= -76.338143 deg)

```
 WGS 84 height: 94.8606 m
|qc - header| position  :  581.0546 m
Observation interval    : 1.0000 seconds
Total satellites w/ obs : 12
NAVSTAR GPS SVs w/o OBS :  1   2   3   4   5   6   7   8  10  11  13  16
                          17  20  23  24  26  28  29  30  31  32
NAVSTAR GPS SVs w/o NAV :
SBAS SVs w/o OBS : 20  21  22  23  24  25  26  27  28  29  30  31
                   32  33  34  35  36  37  38  39  40  41  42
SBAS SVs w/o NAV : 33  38
Rx tracking capability  : 12 SVs
Poss. # of obs epochs   :   1760
Epochs w/ observations  :   1758
Epochs repeated         :      0  (0.00%)
Possible obs >   0.0 deg:  17564
Possible obs >  10.0 deg:  14080
Complete obs >  10.0 deg:  13314
Deleted obs >  10.0 deg:      7
Masked obs <  10.0 deg:    740
Obs w/ SV duplication   :      0  (within non-repeated epochs)
Moving average MP12     : 0.418634 m
Moving average MP21     : 0.443385 m
Points in MP moving avg : 50
Mean S1                 : 47.80 (sd=2.53 n=16776)
Mean S2                 : 41.56 (sd=4.23 n=13314)
No. of Rx clock offsets : 0
Total Rx clock drift    :  0.000000 ms
Rate of Rx clock drift  :  0.000 ms/hr
Avg time between resets : Inf minute(s)
Freq no. and timecode   : 2 11429 010000
Report gap > than       : 10.00 minute(s)
epochs w/ msec clk slip : 0
other msec mp events    : 0 (: 118)   {expect ~= 1:50}
IOD signifying a slip   : >400.0 cm/minute
IOD slips <  10.0 deg*  :     66
IOD slips >  10.0 deg   :     92
IOD or MP slips <  10.0*:     66
IOD or MP slips >  10.0 :     92
 * or unknown elevation
   first epoch    last epoch    hrs   dt  #expt  #have   %   mp1   mp2 o/slps
SUM 11  4 22 16:25 11  4 22 16:54 .4883   1  14080  13314  95  0.42  0.44    145
```

Processing parameters are:
Receiver tracking capability : 12 SVs
Maximum ionospheric rate (L1) : 400.00 cm/min
Report data gap greater than : 10.00 min
Expected rms of MP12 multipath : 65.00 cm
Expected rms of MP21 multipath : 65.00 cm
Multipath slip sigma threshold : 4.00 sigma
% increase in MP rms for C/A | A/S : 100.00 %
Points in MP moving averages : 50
Minimum signal to noise for L1 : 4
Minimum signal to noise for L2 : 4
Elevation mask (cutoff) : 10.00 degrees
Elevation comparison threshold : 25.00 degrees
Orbit path spline fit sample time : 10 min
SVs w/ code data for position try : 5
Width of ASCII summary plot : 72
Data indicators on summary plot : yes
Do ionospheric observable : yes
Do ionospheric derivative : yes
Do multipath observables : yes
Do 1-ms receiver clock slips : yes
Tolerance for 1-ms clock slips : 1.00e-002 ms
Do receiver LLI slips : yes
Do plot file(s) : no

Observations start : 2011 Apr 22 16:25:10.000
Observations end : 2011 Apr 22 16:54:29.000
Observation interval : 1.0000 second(s)

SV	#+hor	<ele>	#+mask	<ele>	#reprt	#compl	L1	L2	P1	P2	C1	C2
G14	1760	47.91	1760	47.91	1755	1755	1755	1755	1755	1755	0	0
G27	1760	35.07	1760	35.07	1744	1744	1744	1744	1744	1744	0	0
G21	1760	18.92	1760	18.92	1496	1493	1496	1493	1496	1493	0	0
G09	1760	51.90	1760	51.90	1756	1756	1756	1756	1756	1756	0	0
G19	1760	5.22	0	90.00	0	0	0	0	0	0	0	0
G22	1760	70.43	1760	70.43	1756	1756	1756	1756	1756	1756	0	0
G12	1760	17.53	1760	17.53	1395	1391	1395	1391	1395	1391	0	0
G15	1760	17.22	1760	17.22	1663	1663	1663	1663	1663	1663	0	0
G18	1760	73.39	1760	73.39	1756	1756	1756	1756	1756	1756	0	0
G25	1724	4.94	0	90.00	0	0	0	0	0	0	0	0
S33*	0	0.00	0	0.00	1739	0	1739	0	0	0	0	0
S38*	0	0.00	0	0.00	1716	0	1716	0	0	0	0	0

* = SV with no NAV info (or not being used)

Obs below mask (10.00 deg) : 4267
Obs reported w/ code | phase : 17588
Obs deleted (any reason) : 4274
Obs complete : 13314

| G | S |
% Obs above mask w/ no L1 : 0.0 0.0
% Obs above mask w/ no L2 : 0.1 -

% Obs above mask w/ no P1 : 0.0 -
% Obs above mask w/ no P2 : 0.1 -

% Obs above mask w/ low L1 S/N: 0.0 0.1
% Obs above mask w/ low L2 S/N: 13.0 -

No. of Rx clock offsets : 0
Total Rx clock drift : 0.000000 ms
Rate of Rx clock drift : 0.000000 ms/hr

elev (deg)	tot slps	<ION rms, m>	5=%	1|m	15=%	2|m	
85 - 90	0	0	0.000000				
80 - 85	0	0	0.000000				
75 - 80	939	0	0.000000				
70 - 75	1320	2	0.000000				
65 - 70	1099	0	0.000000				
60 - 65	152	0	0.000000				
55 - 60	348	0	0.000000				
50 - 55	1364	2	0.000000				
45 - 50	1409	1	0.000000				
40 - 45	421	0	0.000000				
35 - 40	854	1	0.000000				
30 - 35	789	1	0.000000				
25 - 30	129	0	0.000000				
20 - 25	1220	31	0.000000	=====			
15 - 20	2333	24	0.000000	==			
10 - 15	929	30	0.000000	======			
5 - 10	790	65	0.000000	================			
0 - 5	10	1	0.000000	==================			
< 0	0	0	0.000000				

MP12 RMS summary (per SV):

SV	obs>10	# del	<elev>	MP12 rms [m]	slps < 25	L1 rx < 25	L2 rx < 25	slps > 25	L1 rx > 25	L2 rx > 25

```
G14   1755     0   47.89  0.098894     0     0     0     1     2     2
G27   1744     0   35.04  0.119038     0     0     0     1     4     4
G21   1496     3   18.61  1.316790     6    46    46     0     0     0
G09   1756     0   51.87  0.055945     0     0     0     1     1     1
G19      0     0    0.00  0.000000     0     0     0     0     0     0
G22   1756     0   70.42  0.055830     0     0     0     1     1     1
G12   1395     4   18.12  1.376350    10    47    47     0     0     0
G15   1663     0   17.34  0.619383     4    15    15     0     0     0
G18   1756     0   73.38  0.049026     0     0     0     1     1     1
G25      0     0    0.00  0.000000     0     0     0     0     0     0

mean MP12 rms        :  0.418659 m
total mean elevation : 42.85 degrees
# MP12 obs > 10      :  13314
# qc MP12 slips < 25 :     20
# Rvr L1 slips < 25  :    108
# Rvr L2 slips < 25  :    108
# qc MP12 slips > 25 :      5
# Rvr L1 slips > 25  :      9
# Rvr L2 slips > 25  :      9

elev (deg)  tot slps <MP12 rms, m>       5=%       1|m      15=%       2|m
 85 - 90      0      0   0.000000
 80 - 85      0      0   0.000000
 75 - 80    939      0   0.045515  |
 70 - 75   1320      2   0.060272  |
 65 - 70   1099      0   0.048531  |
 60 - 65    152      0   0.037130  |
 55 - 60    348      0   0.034057  |
 50 - 55   1364      1   0.099626  ||
 45 - 50   1409      1   0.075503  ||
 40 - 45    421      0   0.040224  |
 35 - 40    854      1   0.063831  |
 30 - 35    789      0   0.160885  |||
 25 - 30    129      0   0.043057  |
 20 - 25   1220      4   1.273722  ||||||||||||||||||||||||
 15 - 20   2333     10   0.755336  ||||||||||||||
 10 - 15    929      6   1.638666  #|||||||||||||||||||||||||||||
  5 - 10    790     29   2.650962  #######|||||||||||||||||||||||||||||||||||||>
  0 -  5     10      1   0.070504  #==================
    <  0      0      0   0.000000

MP21 RMS summary (per SV):
                              slips  L1 rx  L2 rx  slips  L1 rx  L2 rx
```

SV	obs>10	# del	<elev>	MP21 rms [m]	< 25	< 25	< 25	> 25	> 25	> 25
G14	1755	0	47.89	0.105457	0	0	0	1	2	2
G27	1744	0	35.04	0.093093	0	0	0	1	4	4
G21	1496	3	18.61	1.150643	14	46	45	0	0	0
G09	1756	0	51.87	0.040095	0	0	0	1	1	1
G19	0	0	0.00	0.000000	0	0	0	0	0	0
G22	1756	0	70.42	0.034036	0	0	0	1	1	1
G12	1395	4	18.12	1.901072	13	47	46	0	0	0
G15	1663	0	17.34	0.609290	5	15	15	0	0	0
G18	1756	0	73.38	0.028751	0	0	0	1	1	1
G25	0	0	0.00	0.000000	0	0	0	0	0	0

mean MP21 rms : 0.443416 m
total mean elevation : 42.85 degrees
MP21 obs > 10 : 13314
qc MP21 slips < 25 : 32
Rvr L1 slips < 25 : 108
Rvr L2 slips < 25 : 106
qc MP21 slips > 25 : 5
Rvr L1 slips > 25 : 9
Rvr L2 slips > 25 : 9

elev (deg)	tot slps	<MP21 rms, m>	5=%	1|m	15=%	2|m
85 - 90	0	0	0.000000			
80 - 85	0	0	0.000000			
75 - 80	939	0	0.029223	|		
70 - 75	1320	2	0.031829	|		
65 - 70	1099	0	0.028958	|		
60 - 65	152	0	0.052204	|		
55 - 60	348	0	0.041002	|		
50 - 55	1364	1	0.111141	||		
45 - 50	1409	1	0.052742	|		
40 - 45	421	0	0.043113	|		
35 - 40	854	1	0.036216	|		
30 - 35	789	0	0.128793	|||		
25 - 30	129	0	0.059211	|		
20 - 25	1220	10	1.161950	#||||||||||||||||||||		
15 - 20	2333	13	0.726238	#||||||||||||||		
10 - 15	929	9	2.276403	#|||||||||||||||||||

```
elev (deg)  tot SN1 sig    mean         2|0       4|0       6|0       8|0
 85 - 90      0   0.000    0.000
 80 - 85      0   0.000    0.000
 75 - 80    940   1.763   50.077 #|||||||||||||||||||||||
 70 - 75   1320   1.445   50.657 #|||||||||||||||||||||||
 65 - 70   1099   1.566   50.871 #|||||||||||||||||||||||
 60 - 65    153   4.124   50.630 ##|||||||||||||||||||||||
 55 - 60    349   2.738   50.842 #|||||||||||||||||||||||
 50 - 55   1364   1.514   50.295 #|||||||||||||||||||||||
 45 - 50   1409   1.470   49.865 #|||||||||||||||||||||||
 40 - 45    423   2.470   48.834 #|||||||||||||||||||||||
 35 - 40    854   1.776   47.746 #|||||||||||||||||||||||
 30 - 35    789   1.738   47.119 #|||||||||||||||||||||||
 25 - 30    130   4.187   45.806 ##|||||||||||||||||||||||
 20 - 25   1224   2.044   45.499 #|||||||||||||||||||||||
 15 - 20   2333   1.541   45.507 #|||||||||||||||||||||||
 10 - 15    934   2.349   43.866 #|||||||||||||||||||||||
  5 - 10    797   2.825   42.662 #|||||||||||||||||||||||
  0 -  5     15  10.980   38.983 #####|||||||||||||||
    <  0      0   0.000    0.000
```

S/N L2 summary (per elevation bin):

```
elev (deg)  tot SN2 sig    mean         2|0       4|0       6|0       8|0
 85 - 90      0   0.000    0.000
 80 - 85      0   0.000    0.000
 75 - 80    940   1.808   46.208 #|||||||||||||||||||||||
 70 - 75   1320   1.418   46.495 #|||||||||||||||||||||||
 65 - 70   1099   1.537   46.014 #|||||||||||||||||||||||
 60 - 65    153   3.740   45.489 ##|||||||||||||||||||||||
 55 - 60    349   2.538   45.302 #|||||||||||||||||||||||
 50 - 55   1364   1.620   44.252 #|||||||||||||||||||||||
 45 - 50   1409   1.653   43.064 #|||||||||||||||||||||||
 40 - 45    423   2.133   41.305 #|||||||||||||||||||||||
 35 - 40    854   1.602   40.573 #|||||||||||||||||||||||
 30 - 35    789   1.612   38.773 #|||||||||||||||||||||||
 25 - 30    130   3.538   36.873 ##|||||||||||||||||||||||
 20 - 25   1221   3.017   37.134 ##|||||||||||||||||||||||
 15 - 20   2333   2.450   37.649 #|||||||||||||||||||||||
 10 - 15    930   3.155   35.330 ##|||||||||||||||||||||||
  5 - 10    791   3.621   33.671 ##|||||||||||||||||||||||
  0 -  5     11  10.191   29.257 #####|||||||||||
    <  0      0   0.000    0.000
```

C.2 Quality check Report of GPS Observations for Trajectory 2

```
version: teqc  2015Nov6
 SV+---------|------------|------------|------------|-------------|---------+ SV
 25|oooooo-oooooIIL                                                | 25
 10|oooooo-ooooooooIooooIooooooooooooooIoIIIIIIIIoIoaI   L              | 10
  7|oooooo-ooooooooIIIoIIIooooooooooooooIoIIoIIIIIIIII  LIIIIIIIoooIIIIIILII|  7
  5|oooooo-oooooooooooooooooooooooooooooooooooooooooooooomooooooooooooooooooo-|  5
 21|oooooo-ooooooooooIooIooooooooooooooooooooIIIoIIIIIIIIIIIIIIIIILIIoIIIIooo-| 21
 29|oooooo-oooooooooooooooooooooooooooooooooooooooooooooomooooooooooooooooooo-| 29
  2|oooooo-ooooooooooooooooooooooooooooooooooomoooLIoooIIIIIIoILoooIIIIoIoIIo-|  2
 15|oooooo-ooooooooooooooooooooooooooooooooooIoIIoooIooooooIooooooooooIoo-| 15
 26|oooooo-ooooooooooooooooooooooooooooooooooooooooooomooooIooooooooooooo-| 26
S35|aaaaaa-
aaaaaaaaaLaaaLaaaaaaaaaaaaaaaaaaaaLLLLaaaaLLLLLLaLaaaLLaaLLLLLLLL|S35
S33|aaaaaa-aaaaaaaaaaaaaLaaaaaaaaaaaaaaaaaaaaaLaLLaaaaaLaaLaLLaaaaaaaaaaLaaL|S33
  8| Loooo-ooooooooIIIoIIIooooooooooooooIoIIoIIIIIooooII LIIooooooIoIIoIIoIL-|  8
 18|                                  LILII IL IIoIIooIIIoIIIIIIII| 18
Obs|bccccccccccccccbbbbbbbbbbbbbbbbbbbbbbbbbbbbabccccb999abbbbbbbbbbbbabbbbbb|Obs
Clk|     ^                                                        ^|Clk
   +---------|------------|------------|------------|-------------|---------+
18:32:31.000                                          19:27:38.000
2012 Dec 12                                           2012 Dec 12

**********************
QC of RINEX  file(s) : CPT_DEC12_1_31pm.12o
**********************

4-character ID        :
Receiver type         : NovAtel GPSCard
Antenna type          :

Time of start of window : 2012 Dec 12  18:32:31.000
Time of  end  of window : 2012 Dec 12  19:27:38.000
Time line window length : 55.12 minute(s), ticked every 10.0 minute(s)
Observation interval    : 1.0000 seconds
Total satellites w/ obs : 13
NAVSTAR GPS SVs w/o OBS :  1   3   4   6   9  11  12  13  14  16  17  19
                          20  22  23  24  27  28  30  31  32
SBAS SVs w/o OBS : 20  21  22  23  24  25  26  27  28  29  30  31
                   32  33  34  35  36  37  38  39  40  41  42
Rx tracking capability  : 12 SVs
Poss. # of obs epochs   :  3308
```

Epochs w/ observations : 3306
Epochs repeated : 0 (0.00%)
Complete observations : 33901
Deleted observations : 43
Obs w/ SV duplication : 0 (within non-repeated epochs)
Moving average MP12 : 0.466840 m
Moving average MP21 : 0.651641 m
Points in MP moving avg : 50
Mean S1 : 47.03 (sd=3.46 n=33944)
Mean S2 : 40.39 (sd=5.55 n=27800)
No. of Rx clock offsets : 0
Total Rx clock drift : 0.000000 ms
Rate of Rx clock drift : 0.000 ms/hr
Avg time between resets : Inf minute(s)
Freq no. and timecode : 2 12029 0c0000
Report gap > than : 10.00 minute(s) but < than : 90.00 minute(s)
epochs w/ msec clk slip : 0
other msec mp events : 4 (: 223) {expect ~= 1:50}
IOD signifying a slip : >400.0 cm/minute
IOD slips : 260
IOD or MP slips : 260

	first epoch	last epoch	hrs	dt	#expt	#have	%	mp1	mp2	o/slps
SUM	12 12 12 18:32	12 12 12 19:27	.9183	1	-	33901	-	0.47	0.65	130

Processing parameters are:
Receiver tracking capability : 12 SVs
Maximum ionospheric rate (L1) : 400.00 cm/min
Report data gap greater than : 10.00 min but less than : 90.00 min
Expected rms of MP12 multipath : 65.00 cm
Expected rms of MP21 multipath : 65.00 cm
Multipath slip sigma threshold : 4.00 sigma
% increase in MP rms for C/A | A/S : 100.00 %
Points in MP moving averages : 50
Minimum signal to noise for L1 : 4
Minimum signal to noise for L2 : 4
Width of ASCII summary plot : 72
Data indicators on summary plot : yes
Do ionospheric observable : yes
Do ionospheric derivative : yes
Do multipath observables : yes
Do 1-ms receiver clock slips : yes
Tolerance for 1-ms clock slips : 1.00e-002 ms
Do receiver LLI slips : yes
Do plot file(s) : no

Observations start : 2012 Dec 12 18:32:31.000
Observations end : 2012 Dec 12 19:27:38.000
Observation interval : 1.0000 second(s)

SV	#+hor	<ele>	#+mask	<ele>	#reprt	#compl	L1	L2	P1	P2	C1	C2
G02*	0	0.00	0	0.00	3119	3119	3119	3119	0	3119	3119	0
G10*	0	0.00	0	0.00	2109	2101	2109	2101	0	2101	2109	0
G15*	0	0.00	0	0.00	3261	3261	3261	3261	0	3261	3261	0
G05*	0	0.00	0	0.00	3301	3301	3301	3301	0	3301	3301	0
G21*	0	0.00	0	0.00	2978	2974	2978	2974	0	2974	2978	0
G29*	0	0.00	0	0.00	3300	3300	3300	3300	0	3300	3300	0
G25*	0	0.00	0	0.00	687	687	687	687	0	687	687	0
G07*	0	0.00	0	0.00	2401	2386	2401	2386	0	2386	2401	0
G26*	0	0.00	0	0.00	3300	3300	3300	3300	0	3300	3300	0
S35*	0	0.00	0	0.00	2871	2871	2871	0	0	0	2871	0
S33*	0	0.00	0	0.00	3230	3230	3230	0	0	0	3230	0
G08*	0	0.00	0	0.00	2736	2729	2736	2729	0	2729	2736	0
G18*	0	0.00	0	0.00	651	642	651	642	0	642	651	0

* = SV with no NAV info (or not being used)
Obs reported w/ code | phase : 33944
Obs deleted (any reason) : 43
Obs complete : 33901

		G	S
% Obs	w/ no L1	0.0	0.0
% Obs	w/ no L2	0.2	-
% Obs	w/ no P2	0.2	-
% Obs	w/ no C1	0.0	0.0
% Obs	w/ low L1 S/N:	0.3	0.1
% Obs	w/ low L2 S/N:	31.0	-

No. of Rx clock offsets : 0
Total Rx clock drift : 0.000000 ms
Rate of Rx clock drift : 0.000000 ms/hr

MP12 RMS summary (per SV):

SV	Slips L1 rx obs	# del	<elev>	MP12 rms [m]	L1 rx all	L2 rx all	all
G02*	3119	0	0.00	0.532853	3	31	31
G10*	2109	8	0.00	0.473746	13	31	31
G15*	3261	0	0.00	0.234317	1	11	11
G05*	3301	0	0.00	0.036847	1	1	1

G21* 2978 4 0.00 1.024433 7 55 55
G29* 3300 0 0.00 0.033013 1 1 1
G25* 687 0 0.00 0.322434 0 3 3
G07* 2401 15 0.00 1.283089 35 89 89
G26* 3300 0 0.00 0.063456 1 2 2
G08* 2736 7 0.00 0.632240 24 43 43
G18* 651 9 0.00 1.653790 21 45 45
* = SV with no NAV info (or not being used)

mean MP12 rms : 0.466840 m
MP12 obs : 27800
MP12 slips : 107
Rvr L1 slips : 312
Rvr L2 slips : 312

MP21 RMS summary (per SV):
Slips L1 rx L2 rx
SV obs # del <elev> MP21 rms [m] all all all
G02* 3119 0 0.00 0.360978 5 31 31
G10* 2109 8 0.00 0.688594 14 31 30
G15* 3261 0 0.00 0.326418 1 11 11
G05* 3301 0 0.00 0.062317 1 1 1
G21* 2978 4 0.00 1.320900 8 55 54
G29* 3300 0 0.00 0.045425 1 1 1
G25* 687 0 0.00 0.534082 0 3 3
G07* 2401 15 0.00 2.670451 33 89 82
G26* 3300 0 0.00 0.083037 1 2 2
G08* 2736 7 0.00 0.837369 25 43 45
G18* 651 9 0.00 1.396857 27 45 43
* = SV with no NAV info (or not being used)

mean MP21 rms : 0.651641 m
MP21 obs : 27800
MP21 slips : 116
Rvr L1 slips : 312
Rvr L2 slips : 303

C.3 Quality check Report of GPS Observations for Trajectory 3

```
version: teqc  2015Nov6
 SV+-----------------|--------------------|--------------------|------------+ SV
  7|IIIIIIIIILLLLL___L_____________                                         |  7
 29|LLLLLIIIIIIII----IIIIIIIIIII------III-III-I---I-___L___LLL_____________  | 29
  3|LIIIIIIIIIII---IIIIIIII-I--------MI---2-I---------_L_L___LLLLLLLLLLLLL_|  3
 21|LLLLLII-IIIII---IILLLLLLLLMLIIIIIIIILLI-IL-IILL-LLIILL-IIII-IILIIIILLLL| 21
 26|LLLLL-IIIIIII---IIIII-IIIIIIIIIIMIIIIIIIIIII-III-II-IIIIIIIIIIILLLLLL| 26
 18|LLLLLI-IIIIII---IIILLLLLLLLLLLIIIILIILLLILILIIIL-ILLLLLLLIIII-IILIIIILLLL| 18
 22|LLLLIIIIIIII---IIIIIIIIILIIIIIIIIIIIIII-IIIIIIIIIIIIIIIIIIIIILLLL| 22
  9|LLLI-II-IIIII--MIIIIILLLLLIIIIIIIIIILLLLILMIIIIIIII-ILIIIIIILIIILLLLL|  9
 24|LLLLIIIIIII-I-IIIIILLLLLIII-III-IIILLLLI-I-IIIII-IILLIIIIIIIILLL_LLL| 24
  6|____LLLL_LL_____________                                               |  6
 14|        _____________________-----I--I-I--II----------MIIIIIIIIIIIIILLLL| 14
 19|        _____________________________L_L_____________-------IIIIIIII-IIII| 19
 12|                                      ____________L| 12
-dn|+   ++   +   7761 1     1 1111445221122121325333223312112 1 + 1 +  + ++  |-dn
+dn| 2134cccc7ccccccccc566444346658ccc7cc65575c6ccc7785667455cc5cccc4ccc51111|+dn
+10|899999999888888888888888888899999999999999999999998888888888888887666666|+
10
Pos|oooooooo oo                                                          |Pos
Clk|    ^^^^ ^^^^^^^^           ^^^ ^^     ^ ^^^         ^^ ^^^^ ^^^    |Clk
   +-----------------|--------------------|--------------------|------------+
14:35:27.000                                          16:17:22.000
2007 Jun 15                                           2007 Jun 15

**********************
QC of RINEX  file(s) : CDU_RAWIMUSB.07O
input RnxNAV file(s) : CDU_RAWIMUSB.07N
**********************

4-character ID        :
Receiver type         : NovAtel GPSCard
Antenna type          :

Time of start of window : 2007 Jun 15  14:35:27.000
Time of  end  of window : 2007 Jun 15  16:17:22.000
Time line window length : 101.92 minute(s), ticked every 30.0 minute(s)
  antenna WGS 84 (xyz)  : 853098.5036 -4541209.4436 4382019.2201 (m)
  antenna WGS 84 (geo)  : N  43 deg 40' 25.86"  W  79 deg 21' 38.09"
  antenna WGS 84 (geo)  :   43.673850 deg   280.639420 deg (=  -79.360580 deg)
        WGS 84 height : 96.5472 m
```

```
|qc - header| position  :  6744.3210 m
Observation interval    : 1.0000 seconds
Total satellites w/ obs : 13
NAVSTAR GPS SVs w/o OBS :  1   2   4   5   8  10  11  13  15  16  17  20
                          23  25  27  28  30  31  32
NAVSTAR GPS SVs w/o NAV :
Rx tracking capability  : 12 SVs
Poss. # of obs epochs   :   6116
Epochs w/ observations  :   5667
Epochs repeated         :      0  (0.00%)
Possible obs >   0.0 deg:  64617
Possible obs >  10.0 deg:  50035
Complete obs >  10.0 deg:  28953
 Deleted obs >  10.0 deg:    859
  Masked obs <  10.0 deg:  16513
Obs w/ SV duplication   :      0  (within non-repeated epochs)
Moving average MP12     : 1.202179 m
Moving average MP21     : 1.234892 m
Points in MP moving avg : 50
Mean S1                 : 45.74 (sd=4.07 n=29812)
Mean S2                 : 39.07 (sd=4.71 n=28953)
No. of Rx clock offsets : 0
Total Rx clock drift    :  0.000000 ms
Rate of Rx clock drift  :  0.000 ms/hr
Avg time between resets : Inf minute(s)
Freq no. and timecode   : 2 10022 01c000
Report gap > than       : 10.00 minute(s)
epochs w/ msec clk slip : 0
other msec mp events    : 0 (: 1677)   {expect ~= 1:50}
IOD signifying a slip   : >400.0 cm/minute
IOD slips <  10.0 deg*  :    125
IOD slips >  10.0 deg   :   1054
IOD or MP slips <  10.0*:    131
IOD or MP slips >  10.0 :   1081
 * or unknown elevation
     first epoch    last epoch    hrs   dt  #expt  #have   %   mp1   mp2 o/slps
SUM 07  6 15 14:35 07  6 15 16:17 1.574   1  50035  28953  58  1.20  1.23     27



Processing parameters are:
Receiver tracking capability       : 12 SVs
Maximum ionospheric rate (L1)      : 400.00 cm/min
Report data gap greater than       : 10.00 min
```

Expected rms of MP12 multipath : 65.00 cm
Expected rms of MP21 multipath : 65.00 cm
Multipath slip sigma threshold : 4.00 sigma
% increase in MP rms for C/A | A/S : 100.00 %
Points in MP moving averages : 50
Minimum signal to noise for L1 : 4
Minimum signal to noise for L2 : 4
Elevation mask (cutoff) : 10.00 degrees
Elevation comparison threshold : 25.00 degrees
Orbit path spline fit sample time : 10 min
SVs w/ code data for position try : 5
Width of ASCII summary plot : 72
Data indicators on summary plot : yes
Do ionospheric observable : yes
Do ionospheric derivative : yes
Do multipath observables : yes
Do 1-ms receiver clock slips : yes
Tolerance for 1-ms clock slips : 1.00e-002 ms
Do receiver LLI slips : yes
Do plot file(s) : yes

Observations start : 2007 Jun 15 14:35:27.000
Observations end : 2007 Jun 15 16:17:22.000
Observation interval : 1.0000 second(s)

SV	#+hor	<ele>	#+mask	<ele>	#reprt	#compl	L1	L2	P1	P2	C1	C2
G09	6116	48.51	6116	48.51	4668	4573	4668	4573	0	4573	4668	0
G29	5877	17.39	4100	22.79	1540	1480	1540	1480	0	1480	1540	0
G24	6116	28.09	5513	30.29	3425	3352	3425	3352	0	3352	3425	0
G21	6116	59.61	6116	59.61	5034	4962	5034	4962	0	4962	5034	0
G26	6116	26.73	5597	28.42	2590	2465	2590	2465	0	2465	2590	0
G18	6116	69.95	6116	69.95	5337	5308	5337	5308	0	5308	5337	0
G22	6116	38.45	6116	38.45	3978	3860	3978	3860	0	3860	3978	0
G07	2532	7.05	742	12.19	442	247	442	247	0	247	442	0
G03	6116	10.40	4236	11.53	692	644	692	644	0	644	692	0
G06	1747	4.90	0	90.00	0	0	0	0	0	0	0	
G14	5314	16.29	3682	21.30	1358	1335	1358	1335	0	1335	1358	0
G19	5321	7.37	1701	10.36	748	727	748	727	0	727	748	0
G12	1014	2.53	0	90.00	0	0	0	0	0	0	0	

Obs below mask (10.00 deg) : 1524
Obs reported w/ code | phase : 31336
Obs deleted (any reason) : 2383

Obs complete : 28953

| G |
% Obs above mask w/ no L1 : 0.0
% Obs above mask w/ no L2 : 2.9

% Obs above mask w/ no P2 : 2.9
% Obs above mask w/ no C1 : 0.0

% Obs above mask w/ low L1 S/N: 4.9
% Obs above mask w/ low L2 S/N: 22.5

No. of Rx clock offsets : 0
Total Rx clock drift : 0.000000 ms
Rate of Rx clock drift : 0.000000 ms/hr

elev (deg)	tot slps	<ION rms, m>	5=%	1|m	15=%	2|m	
85 - 90	0	0	0.000000				
80 - 85	0	0	0.000000				
75 - 80	3127	52	0.000000				===
70 - 75	1322	11	0.000000				=
65 - 70	1439	12	0.000000				=
60 - 65	919	33	0.000000				=======
55 - 60	3491	67	0.000000				===
50 - 55	2328	41	0.000000				===
45 - 50	2431	82	0.000000				======
40 - 45	2171	100	0.000000				=========
35 - 40	2240	66	0.000000				=====
30 - 35	2925	81	0.000000				=====
25 - 30	1779	103	0.000000				==========
20 - 25	1594	102	0.000000				==========
15 - 20	728	38	0.000000				==========
10 - 15	2450	266	0.000000				==========
5 - 10	1435	124	0.000000				==========
0 - 5	7	1	0.000000				==========
< 0	0	0	0.000000				

MP12 RMS summary (per SV):

SV	obs>10	# del	<elev>	MP12 rms [m]	slips < 25	L1 rx < 25	L2 rx < 25	slips > 25	L1 rx > 25	L2 rx > 25
G09	4668	95	48.79	0.801163	0	0	0	77	4668	4668
G29	1540	60	28.56	3.920772	31	540	540	8	1000	1000
G24	3425	73	31.94	1.006811	28	1091	1091	43	2334	2334
G21	5034	72	59.14	0.858058	0	0	0	63	5034	5034

```
G26   2590    125   33.19  1.546442     46    698    698     61   1892   1892
G18   5337     29   70.48  0.766362      0      0      0     39   5337   5337
G22   3978    118   39.43  1.497777     14    748    748    112   3230   3230
G07    442    195   12.41  1.640084     39    442    442      0      0      0
G03    692     48   11.24  1.692441     59    692    692      0      0      0
G06      0      0    0.00  0.000000      0      0      0      0      0      0
G14   1358     23   27.19  1.093109     21    242    242      5   1116   1116
G19    748     21   10.42  1.505426     33    748    748      0      0      0
G12      0      0    0.00  0.000000      0      0      0      0      0      0

mean MP12 rms        :  1.202259 m
total mean elevation : 45.87 degrees
# MP12 obs > 10      :  28953
# qc MP12 slips < 25 :    271
# Rvr L1 slips < 25  :   5201
# Rvr L2 slips < 25  :   5201
# qc MP12 slips > 25 :    408
# Rvr L1 slips > 25  :  24611
# Rvr L2 slips > 25  :  24611

elev (deg)  tot slps <MP12 rms, m>        5=%       1|m      15=%       2|m
 85 - 90      0    0   0.000000
 80 - 85      0    0   0.000000
 75 - 80   3127   22   0.760252  #|||||||||||||
 70 - 75   1322    7   0.568049  #|||||||||
 65 - 70   1439   13   0.404129  #|||||||
 60 - 65    919   26   0.825984  #####||||||||||
 55 - 60   3491   32   0.751374  #|||||||||||||
 50 - 55   2328   31   0.682548  ##||||||||||||
 45 - 50   2431   63   0.862933  #####||||||||||
 40 - 45   2171   52   1.841568  ####||||||||||||||||||||||||||||||
 35 - 40   2240   42   1.158089  ###||||||||||||||||||
 30 - 35   2925   65   0.641181  ####|||||||||
 25 - 30   1779   55   1.556587  ######|||||||||||||||||||||||
 20 - 25   1594   76   1.601689  #########|||||||||||||||||||||||
 15 - 20    728   28   2.926426  #######||||||||||||||||||||||||||||||||||>
 10 - 15   2450  167   3.000081  ##############|||||||||||||||||||||||||||||>
  5 - 10   1435   78   3.950465  ##########||||||||||||||||||||||||||||||>
  0 -  5      7    0   0.522894  |||||||||
    <  0      0    0   0.000000

MP21 RMS summary (per SV):
                                slips  L1 rx  L2 rx  slips  L1 rx  L2 rx
 SV  obs>10  # del <elev> MP21 rms [m]  < 25   < 25   < 25   > 25   > 25   > 25
```

```
G09   4668    95   48.79  1.070876     0     0     0    98   4668   4573
G29   1540    60   28.56  1.868957    53   540   489     9   1000    991
G24   3425    73   31.94  1.504211    36  1091  1075    49   2334   2277
G21   5034    72   59.14  0.858834     0     0     0    74   5034   4962
G26   2590   125   33.19  1.584078    55   698   623    65   1892   1842
G18   5337    29   70.48  0.769767     0     0     0    55   5337   5308
G22   3978   118   39.43  1.309869    15   748   748   140   3230   3112
G07    442   195   12.41  3.441053    37   442   247     0      0      0
G03    692    48   11.24  2.814746    64   692   644     0      0      0
G06      0     0    0.00  0.000000     0     0     0     0      0      0
G14   1358    23   27.19  1.319152    25   242   222     8   1116   1113
G19    748    21   10.42  1.814524    40   748   727     0      0      0
G12      0     0    0.00  0.000000     0     0     0     0      0      0

mean MP21 rms        :  1.234983 m
total mean elevation : 45.87 degrees
# MP21 obs > 10      :  28953
# qc MP21 slips < 25 :    325
# Rvr L1 slips < 25  :   5201
# Rvr L2 slips < 25  :   4775
# qc MP21 slips > 25 :    498
# Rvr L1 slips > 25  :  24611
# Rvr L2 slips > 25  :  24178

elev (deg)  tot slps <MP21 rms, m>       5=%       1|m      15=%       2|m
 85 - 90      0    0   0.000000
 80 - 85      0    0   0.000000
 75 - 80   3127   34   0.931987  ##||||||||||||||||
 70 - 75   1322    8   0.322927  #|||||
 65 - 70   1439   13   0.263311  #||||
 60 - 65    919   29   0.928895  ######||||||||||||
 55 - 60   3491   46   0.740895  ##||||||||||||
 50 - 55   2328   38   0.873441  ###||||||||||||||
 45 - 50   2431   73   0.941436  ######||||||||||||
 40 - 45   2171   67   1.785984  ######||||||||||||||||||||||||||||
 35 - 40   2240   52   0.977209  ####||||||||||||||
 30 - 35   2925   79   0.693091  #####|||||||||
 25 - 30   1779   59   2.084945  ######||||||||||||||||||||||||||||||||
 20 - 25   1594   92   1.441668  ############||||||||||||||||||
 15 - 20    728   36   1.053253  #########||||||||||||
 10 - 15   2450  197   2.756957  ################||||||||||||||||||||||||||||>
  5 - 10   1435   97   3.583705  ##############|||||||||||||||||||||||||||||>
  0 -  5      7    0   1.797734  ||||||||||||||||||||||||||||||||
    <  0      0    0   0.000000
```

S/N L1 summary (per elevation bin):

elev (deg)	tot SN1 sig	mean	2|0	4|0	6|0	8|0
85 - 90	0	0.000	0.000			
80 - 85	0	0.000	0.000			
75 - 80	3146	2.441	47.776	#		
70 - 75	1327	1.681	48.405	#		
65 - 70	1442	2.373	48.431	#		
60 - 65	943	3.428	47.907	##		
55 - 60	3543	2.651	48.549	#		
50 - 55	2344	2.206	48.231	#		
45 - 50	2505	3.314	46.887	##		
40 - 45	2236	3.173	46.664	##		
35 - 40	2298	2.637	45.866	#		
30 - 35	2992	2.532	44.818	#		
25 - 30	1835	2.637	43.601	#		
20 - 25	1660	3.406	41.973	##		
15 - 20	796	3.256	40.773	##		
10 - 15	2745	3.188	38.274	##		
5 - 10	1516	2.590	37.360	#		
0 - 5	8	12.944	31.875	######		
< 0	0	0.000	0.000			

S/N L2 summary (per elevation bin):

elev (deg)	tot SN2 sig	mean	2|0	4|0	6|0	8|0
85 - 90	0	0.000	0.000			
80 - 85	0	0.000	0.000			
75 - 80	3128	2.687	43.411	#		
70 - 75	1322	2.236	43.120	#		
65 - 70	1439	2.352	42.470	#		
60 - 65	919	4.237	41.589	##		
55 - 60	3491	2.740	42.125	#		
50 - 55	2330	2.627	40.896	#		
45 - 50	2432	3.894	39.869	##		
40 - 45	2171	3.537	38.914	##		
35 - 40	2241	2.910	37.938	#		
30 - 35	2926	2.707	37.300	#		
25 - 30	1779	2.942	35.861	#		
20 - 25	1595	3.912	34.370	##		
15 - 20	728	3.784	32.780	##		
10 - 15	2452	4.286	31.816	##		
5 - 10	1438	4.570	29.429	##		
0 - 5	8	10.501	22.375	#####		
< 0	0	0.000	0.000			

Bibliography

Abdel-salam, M. (2005). Precise Point Positioning Using Un-differenced Code and Carrier Phase Observations. PhD Thesis. Department of Geomatics Engineering, University of Calgary, Canada.

Aggarwal, P., Gu, D., Nassar, S., Syed, Z., and El-Sheimy, N. (2007). Extended Particle Filter (EPF) for INS/GPS Land Vehicle Navigation Applications. *Proceedings ION GNSS*, 2619-2626. Fort Worth, TX.

Aggarwal, P. Z-S. (2010). *MEMS-based Integrated Navigation*. Norwood: Archech House.

Aggarwal, P., Syed, Z., and El-Sheimy, N. (2008). Hybrid Extended Particle Filter (HEPF) for Integrated Civilian Navigation System. *Proceedings of IEEE/ION PLAN*, 984-992. Monterey, CA.

Ashby, N., and J. J. Spilker, Jr. (1996). Introduction to Relativity Effects on the Global Positioning System. In: *Global Positioning System: Theory and Applications, Volume II, B*. Parkinson and J. J. Spilker, Jr. (eds.). Washington, D.C.: American Institute of Aeronautics and Astronautics.

Ashby, N., and M. Weiss (March 1999) Global Positioning System Receivers and Relativity: National Institute of Standards and Technology (NIST) Technical Note 1385. Boulder, CO.

ARINC Research Corporation (2004). NAVSTAR GPS Space Segment/Navigation User Interfaces, Interface Specification, IS-GPS-200D (Public Release Version). ARINC Research Corporation: Fountain Valley, CA.

Banville, S., Santerre, R., Cocard, M., Langley, R. (2008). Satellite and Receiver Phase Bias Calibration for Undifferenced Ambiguity Resolution. *Proceedings of ION NTM*. San Diego, CA.

Bisnath, S., and Gao, Y. (2009). Current State of Precise Point Positioning and Future Prospects and Limitations. *International Association of Geodesy Symposia* 133. Springer-Verlag: Berlin, Heidelberg.

Brown, R. G. and Hwang, P. Y. C. (1992). *Introduction to Random Signals*. New York: Wiley.

CarChip OBD II-based vehicle data logger and software (2013). Davis Instruments.

Carvalho, H., Del Moral, P., Monin, A., and Salut, G. (1997). Optimal Nonlinear Filtering in GPS/INS integration. *IEEE Transactions on Aerospace and Electronic Systems* 33:3. 835-50.

Chiang, K.-W., Noureldin, A., and El-Sheimy, N. (2004). A New Weights Updating Method for Neural Networks Based INS/GPS Integration. *Measurement Science and Technology* 15. 2053-61.

Chiang, K.-W. and El-Sheimy, N. (2002). INS/GPS integration using neural networks for land vehicle navigation applications. *Proceedings of the ION GPS*. 535-44. Portland, OR.

Chiang, K.-W., Noureldin, A., and El-Sheimy, N. (2006). The Utilization of Artificial Neural Networks for Multi-Sensor System Integration in Navigation and Positioning Instruments. *IEEE Transactions on Instrumentation and Measurement* 55. 1606-15.

Collins, P., S. Bisnath, F. Lahaye, and P. Heroux (2010). Undifferenced GPS Ambiguity Resolution Using the Decoupled Clock Model and Ambiguity Datum Fixing. *Journal of Navigation* 57:2. 123-35.

Colombo, O.L., Sutter, A.W. and Evans, A.G. (2004). Evaluation of Precise, Kinematic GPS Point Positioning. *Proceedings of ION GNSS*. 2667-75. Long Beach, CA.

Conley, R. et al. (2006). Performance of Stand-Alone GPS. *Understanding GPS: Principles and Applications (second edition)*. Ed.: Kaplan, E.D. ArtechHouse Mobile Communications Series.

Crespillo, O. G., Heirich O., and Lehner A. (2014). "Bayesian GNSS/IMU tight integration for precise railway navigation on track map" IEEE/ION Position, Location and Navigation Symposium - PLANS 2014, DOI: 10.1109/PLANS.2014.6851465

Crossbow (2007). IMU User's Manuals — Models IMU300CC, IMU400CC, IMU400CD. Crossbow Technology Inc.

Davis instruments (2013) CarChip OBDII-based vehicle data logger and software

Dissanayake, G. and Sukkarieh, S. (2001). The aiding of a low-cost strapdown inertial measurement unit using vehicle model constraints for land vehicle applications. *IEEE Trans. Robot. Autom.* 17:5. 731-47.

El-Diasty, M. (2014). Development of Real-Time PPP-Based GPS/INS Integration System Using IGS Real-Time Service for Hydrographic Surveys., J. Surv. Eng., 10.1061/(ASCE)SU.1943-5428.0000150, 05015005

El-Rabbany, A. (2006). *Introduction to GPS: The Global Positioning System*. Artech House: Norwood, MA.

El-Sheimy, N., Chiang, K.-W., and Noureldin, A. (2006). The utilization of artificial neural networks for multisensor system integration in navigation and positioning instruments. *IEEE Transactions on Instrumentation and Measurement* 55:5. 1606-15.

Elsobeiey, M. and El-Rabbany, A. (2010a). Rigorous modeling of GPS residual errors for precise point positioning. Canadian Geomatics Conference and Symposium of Commission I, ISPRS.

Elsobeiey, M. and El-Rabbany, A. (2010b). On stochastic modeling of the modernized global positioning system (GPS) L2C signal. *Journal of Measurement Science and Technology.*

Elsobeiey, Mohamed E., (2012). An Improved Model for Precise Point Positioning with Modernized Global Positioning System. Ryerson University, Theses and dissertations. Paper 1324.

Estey, L., H. and Meertens, C., M. (1999). TEQC: The Multi_Purpose Toolkit for GPS/GLONASS dat.

Farrell, J. A. (2008). *Aided Navigation: GPS with High Rate Sensors*. McGraw-Hill Professional.

Farrell, J. A. (1998*). The Global Positioning System & Inertial Navigation*. New York, NY: McGraw-Hill.

Gao, Y., Chen, K. (2004). Performance Analysis of Precise Point Positioning Using Real-Time Orbit and Clock Products. *Journal of Global Positioning Systems* 3:1-2. 95-100.

Gao, Y., Shen, X. (2001). Improving Ambiguity Convergence in Carrier Phase-Based Precise Point Positioning. *Proceedings of ION GPS*. Salt Lake City, UT.

Gao, Z., Zhang H., Ge M., Niu X., Shen W., Wickert J. and Schuh H. (2015). Tightly Coupled Integration of Ionosphere-Constrained Precise Point Positioning and Inertial Navigation Systems, Sensors 2015, 15, 5783-5802; doi:10.3390/s150305783

Ge, M., Gendt, G., Rothacher, Shi, M. C. and Liu, J. (2008). Resolution of GPS Carrier-phase Ambiguities in Precise Point Positioning (PPP) with Daily Observations. *Journal of Geodesy* 82. 389-399.

Gelb, Ed., A. *Applied Optimal Estimation*. Cambridge, Mass.: M.I.T. Press.

Georgy, J., F., W. (2010) Advanced Nonolinear Techniques for Low Cost Land Vehicle Navigation. PhD thesis, Electrical and Computer Engineering Department Queen's University, Kingston, ON.

Georgy, J., Norueldin, A., Korenberg, M., and Bayoumi, M. (2010). Low-Cost Three Dimensional Navigation Solution for RISS/GPS Integration Using Mixture Particle Filter. IEEE Transaction On Vehicular Technology 59:2.

Georgy, J., Noureldin, A., and Bayoumi, M. (2009). Mixtture Particle Filter for Low Cost INS/Odometer/GPS Integration in Land Vehicles. IEEE.

Gleason, S., Gebre-Egziabher, D. (2009). *GNSS Applications and Methods*. Artech House, Norwood, MA.

Grewal, M. S., Weill, L. R., and Andrews, A. P. (2007). *Global Positioning Systems, Inertial Navigation, and Integration (2nd Edition)*. John Wiley & Sons, Inc.

Grewal, S., and Andrews, A., P. (2008). *Kalman Filtering: Theory and Practice Using MATLAB*. Oxford: Wiley.

Gurney, K. (2003). *An Introduction to Neural Networks*. Taylor & Francis: London, UK.

Hatch, R. (May-June 1995). Relativity and GPS-I. *Galilean Electrodynamics* 6:3. 52–7.

Hay, C., and J. Wong (January 2000). Enhancing GPS: Tropospheric Delay Prediction at the Master Control Station. *GPS World* 11:1. 56-62.

Héroux, P., .Gao, Y., Kouba, J., Lahaye, F.1, Mireault, Y. , Collins, P., Macleod, K., Tétreault, P. and Chen K. (2004). Products and Applications for Precise Point Positioning - Moving Towards Real-Time. ION GNSS 17th International Technical Meeting of the Satellite Division. Long Beach, CA.

Hofmann-Wellenhof, B., Lichtenegger, H., and Collins, J. (2001). *Global Positioning System: Theory and Practice*. Springer-Verlag Wien, NY.

Ibrahim, H. E., and El-Rabbany, A. (2009) Assessment and Implementation of NOAA NWP-Based Tropospheric Correction Model. Proceedings of the IEEE Toronto International Conference — Science and Technology for Humanity. 316-21. (CD-ROM.)

IGS (2016) International GNSS Service (http://www.igs.org/products), accessed May 13, 2016

Iqbal, U. and Noureldin, A. (2009). Integrated Reduced Inertial Sensor System/GPS for Vehicle Navigation: VDM verlag.

Iqbal, U., Ukou, F. and Noureldin, A. (2008). An Integrated Reduced Inertial Sensor System-RISS/GPS for Land Vehicles. In proceeding of IEEE/ION Position, Location and Navigation Symposium. Monterey, CA. 1014-21.

Karamat, T. B. (2009). Implementation of a Tightly Coupled INS/GPS Integration for Land Vehicle Navigation. M.Sc. thesis, Electrical and Computer Engineering Department, Royal Military College of Canada. Kingston, ON.

Karamat, T. B. (2014). Improved Land Vehicle Navigation and GPS Integer Ambiguity Resolution using Enhanced Reduced-IMU/GPS Integration. Queens University, Theses and dissertations,

Karamat, T. B., Atia M. M., Noureldin A. (2014). “Performance Analysis of Code-Phase-Based Relative GPS Positioning and Its Integration with Land Vehicle’s Motion Sensors” IEEE Sensors Journal, doi: 10.1109/JSEN.2014.2324535

Kjørsvik, N. S., Gjevestadb J. G., O., Brøstea E, Gadec K, Hagenc (2010). "Tightly coupled precise point positioning and inertial navigation systems." International calibration and orientation workshop EuroCOW.

Kouba, J., Héroux, P. (2000). GPS Precsie Point Positioining Uisng IGS Orbit Products. *GPS Solutions* 5:2.

Leandro, R.F. and Santos, M.C. (2006). Wide Area Based Precise Point Positioning. Proceedings of ION GNSS. Fort Worth, TX.

Leick, F. (2004). *GPS Satellite Surveying (3rd Edition)*. John Wiley & Sons: USA.

Louis, H., E. and Charles, M., M. (1999). TEQC: The Multi-Purpose Toolkit for GPS/GLNASS data. *GPS Solutions* 3:1. 42-9.

Misra, P. and Angie, P. (2001). *Global Positioning System: Signal, Measurements and Performance*. Ganga-Jumuna Press.

Noureldin, A. (2007). EE523: Course Notes on Mobile Multi-Sensor Systems Integration. Kingston, ON, Canada: Royal Military College. 2007.

Noureldin, A. (2009) EE 523 – Integrated Navigation Systems Course Lecture Notes, Electrical and Computer Engineering Department. Royal Military College of Canada.

Noureldin, A., El-Shafie, A., and Taha, M. R. (2007b). Optimizing neuro-fuzzy modules for data fusion of vehicular navigation systems using temporal cross-validation. *Engineering Applications of Artificial Intelligence* 20. 49-61.

Noureldin, A., Irvine-Halliday, D. and Mintchev, M. (2002). Accuracy Limitations of FOG-Based Continuous Measurement-While-Drilling Surveying Instruments for Horizontal Wells. *IEEE Trans, Instrument Measurement* 51:6. 1177-91.

Noureldin, A., McGaughey, D., Armstrong, J., and Johnston, C. (2007a). Accuracy Enhancement of Inertial Sensors Utilizing High Resolution Spectral Analysis. NavINST, Royal Military College of Canada, Kingston, ON.

Noureldin, A. (2007) EE523: Course Notes on Mobile Multi-Sensor Systems Integration. Royal Military College, Kingston, ON.

Noureldin, A., Osman, A., and El-Sheimy, N. (2004). A Neuro-Wavelet Method for Multi-Sensor System Integration for Vehicular navigation. *Journal of Measurement Science and Technology* 15:2. 404-12.

Noureldin, A., Karamat, T. B., Eberts, M. D, El-Shafie, A. (2009). Performance Enhancement of MEMS Based INS/GPS Integration for Low Cost Navigation Applications. *IEEE Transactions on Vehicular Technology* 58:3. 1077-96.

Novatel (2005), OEM4 Family User Manual OM-20000046 Vol-1 Rev 19. NovAtel Inc, vol. 1.

Novatel (2005), SPAN Technology User Manual OM-20000062 Rev 7. NovAtel Inc,

Obradovic, D., Lenz, H., and Schupfner, M. (2007). Fusion of sensor data in Siemens car navigation system. *IEEE Trans. Veh. Technol*. 56:1. 43-50.

Parkinson, B. W. and Spilker, J. J. (1996). Global Positioning System: Theory and Applications. 2: AIAA.

Perreault, J. M. A., Iqbal, U., Okou, A. F., and Noureldin, A. (2008). RISS/GPS Integration Utilizing an Augumented KF/NN Module. *European Journal of Navigation 6.* 8-13.

Ristic, B., Arulaoalam, S., and Gordon, N. (2004). *Beyond the Kalman Filter: Particle Filters for Tracking Applications*. Artech House: Boston, USA.

Semeniuk, L. and Noureldin, A. (2006). Bridging GPS outages using neural networks estimates of INS position and velocity errors. *Measurement Science and Technology* 17.

Sharaf, R. and Noureldin, A. (2007). Sensor integration for satellite-based vehicular navigation using neural networks. *IEEE Transactions on Neural Networks* 18:2. 589-94.

Shin, E-H. (2005). Estimation Techniques for Low-Cost Inertial Navigation. Ph.D. Thesis, Department of Geomatics Engineering, University of Calgary. Calgary, AB.

Shin, E-H., and Schenzinger, B. (2009). Initially Aided Precise Point Positioning. Proceedings of the ION GNSS 2009. Savannah, GA.

Simon, D. (2006). *Optimal State Estimation: Kalman, H Infinity, and Non-linear Approaches*. Hoboken, N.J; Chichester: John Wiley & Sons, Inc.

Teunissen, P. J. G., Kleusberg, A. (eds.) (1998). *GPS for Geodesy (2nd Edition)*. New York: Springer-Verlag.

Wang, M. and Gao, Y. (2007). An Investigation on GPS Receiver Initial Phase Bias and Its Determination. Proceedings of ION NTM. San Diego, California. 873-80.

Wang, M., and Gao, Y. (2007b). An Investigation on GPS Receiver Initial Phase Bias and Its Determination. Proceedings of ION NTM. San Diego, CA.

Witchayangkoon, B. (2000). Elements of GPS Precise Point Positioning. PhD Thesis, Spatial Information Science and Engineering, University of Maine. 265.

Yi, Y. and Grejner-Brzezinska, D.A. (2006a). Tightly-coupled GPS/INS integration using unscented Kalman filter and particle filter. Proceedings ION GNSS. Fort Worth, TX. 2182-91.

Yi, Y. and Grejner-Brzezinska, D.A. (2006b). Performance comparison of the non-linear Bayesian filters supporting GPS/INS integration. Proceedings ION NTM. Monterey, CA. 977-83.

Zhang, and Gao, Y., (2008). Integration of INS and Un-differenced GPS Measurements for Precise Positioning and Attitude Determination. *Journal of Navigation* 61:1, pp. 87-97.

Zhong, P., X. Ding, D. Zheng and W. Chen (2007). Adaptive wavelet transform based on cross-validation method and its application to GPS multipath mitigation. *GPS Solution*. Springer-Verlag 12:2. 109-17.

Zumberge, J.F., Heflin, M.B., Jefferson, D.C., Watkins, M.M. and Webb, F.H. (1997). Precise Point Processing for the Efficient and Robust Analysis of GPS Data from Large Networks. *Journal of Geophysical Research* 102:B3. 5005-17.